Design Modelling

Oruru Catchment (M09)

Northland Regional Council

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Project Name	Oruru Catchment (M09)
Client	Northland Regional Council
Client Project Manager	Sher Khan & Matt De Boer
Water Technology Project Manager	Bertrand Salmi
Water Technology Project Director	Ben Hughes
Authors	Alvin Mingjun Li, Lachlan Inglis
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15 Business Park Drive
Notting Hill VIC 3168

Telephone	(03) 8526 0800
Fax	(03) 9558 9365
ACN	093 377 283
ABN	60 093 377 283

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Certified System Environment ISO 14001 SAI GLOBAL
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1 PROJECT OVERVIEW

Overview

Water Technology was commissioned by Northland Regional Council (NRC) to undertake a region-wide flood modelling study. The study area encompassed the entire Northland Regional Council area which covers an area of over 12,500 km^2, with the exclusion offshore islands. The aim of this project was to map riverine flood hazard zones across the entire Northland region and update existing flood intelligence.

Modelling approach

This project used a 2D Direct Rainfall (also known as Rain on Grid) approach for hydraulic modelling and has provided flood extents for a defined range of design storms. The hydraulic modelling software TUFLOW was used. TUFLOW is a widely used software package suitable for the analysis of flooding. TUFLOW routes overland flow across a topographic surface (2D domain) to create flood extent, depth, velocity and flood hazard outputs that can be used for planning, intelligence and emergency response. The latest release of TUFLOW offers several recent advanced modelling techniques to improve modelling accuracy which where practical, were tested and adopted in this project.

This study delineated and modelled 19 catchments, shown in Figure 1-1. To validate the adopted methodology and model parameters used in the design modelling, 9 catchments were calibrated against recent (and historic) flood events. The calibration/validation methodology is documented in a standalone report *NRC Riverine Flood Mapping - Calibration Report – R01* and is referred to throughout this document as the *Calibration Report*.

This report documents the design modelling methodology for Oruru Catchment (M09), noting that this catchment was not calibrated.

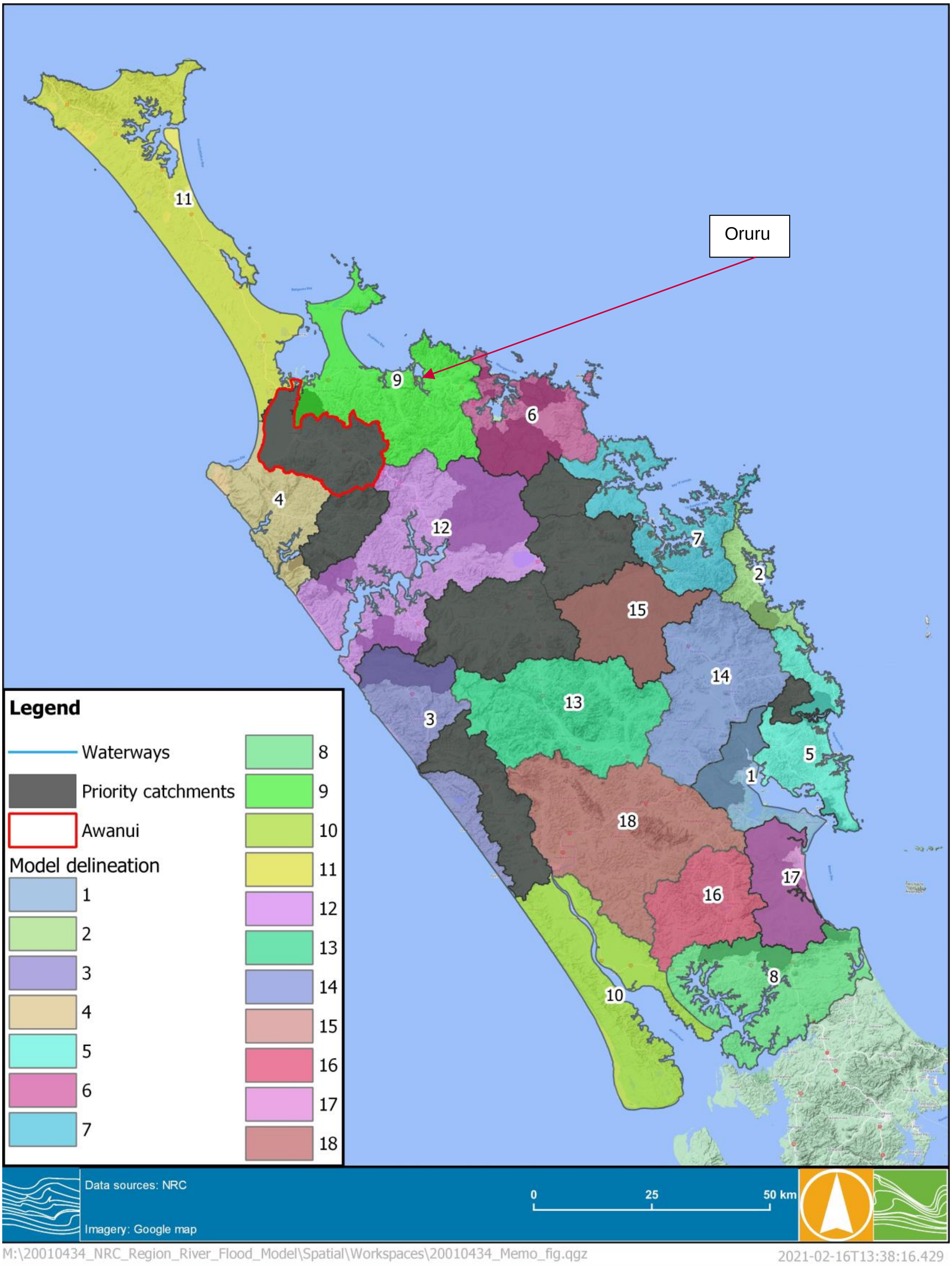

FIGURE 1-1 MODEL DELINEATION

2 STUDY AREA

The model 09 catchment is coastal catchment, covering a total area of approximately 735 km^2. The Oruatiti, Oruru and Pairatahi Rivers are the major waterways within the catchment with tributaries joining them before discharging into Doubtless Bay or Rangauru Bay. Figure 2-1 displays the study area of the catchment model 09.

FIGURE 2-1 STUDY AREA

3 DESIGN MODELLING

3.1 Overview

A hydraulic model (TUFLOW) of the Oruru catchment (M09) was constructed to model overland flooding. A range of storm durations were run and results for each Annual Exceedance Probability (AEP) event were enveloped to ensure the critical duration was well represented across each part of the study area. The merged results captured the maximum flood level and depth of the range of design event durations modelled.

Table 3-1 and the following sections detail the key modelling information used in the development of the hydraulic model.

TABLE 3-1 KEY MODELLING INFORMATION

Terrain data	NRC 1m LiDAR without filling of sinks but includes the "burning of creek alignments' through embankments
Model type	Direct rainfall model
Model build	Build: 2020-10-AA-iSP-w64
Rainfall	See Sections 3.2.1 and 3.2.4
Losses	See Section 3.2.3
Boundaries	See Section 3.2.4
Modelling solution scheme	TUFLOW HPC (adaptive timestep)
Modelling hardware	GPU
Modelling technique	Sub-grid-sampling (SGS)
Model grid size	10m with 1m SGS

3.2 Model Parameters

A range of model parameters were adopted, based on the calibration of catchments (i.e. M03, M06 and M07) in the Far North region. Details of these are outlined below.

3.2.1 Rainfall Intensity-Duration-Frequency

Design rainfall totals for durations from 10 minute up to 120 hours were developed for design modelling. This was undertaken at 179 rainfall gauge sites across the wider study area. These Intensity-Duration-Frequency (IDF) tables were developed by NIWA through the High Intensity Rainfall Design System (HIRDSV4)[1]. A range of magnitude events from 1 in 1.58 ARI through to 1 in 250 ARI along with climate change predictions (Regional Concentration Pathway 4.6, 6 & 8.5) up to 2100. For this catchment, 9 rainfall gauges were used with a spatially weighted grid of rainfall totals created for design modelling. Figure 3-1 shows the 12-hour cumulative rainfall grid for the 1% AEP event along with the rainfall gauge locations used to create the grid.

[1] Accessed via https://hirds.niwa.co.nz/

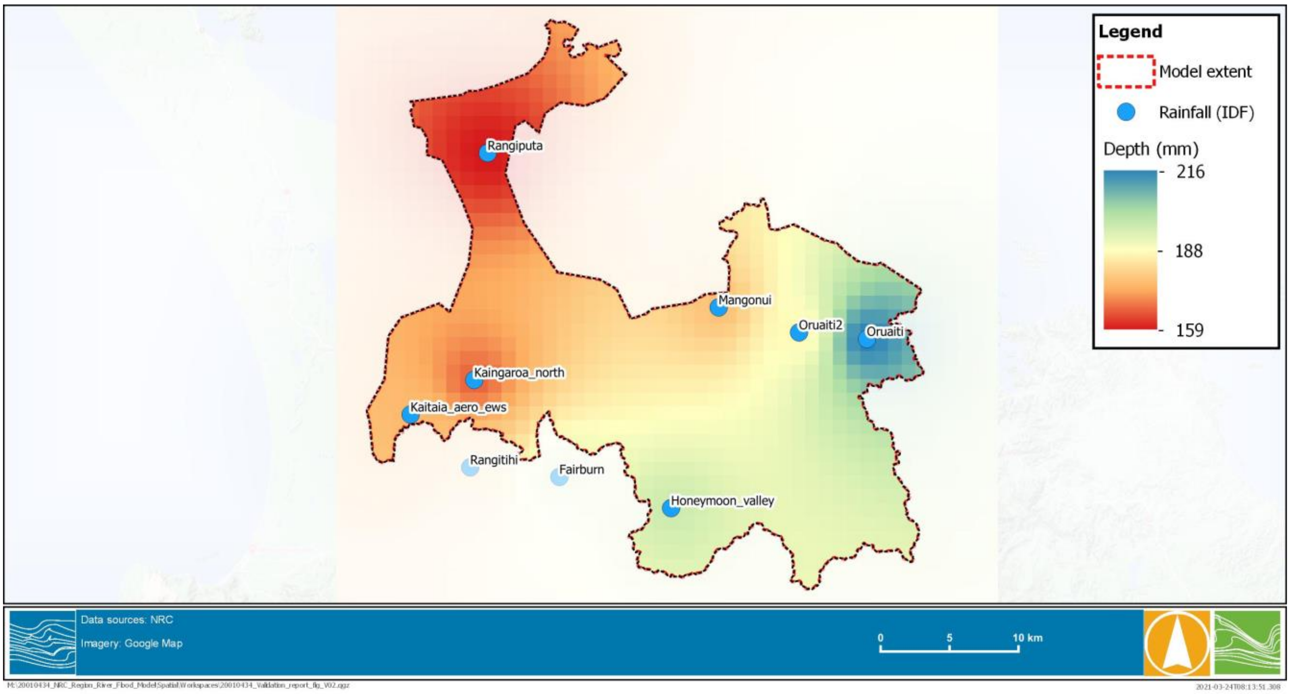

FIGURE 3-1 EXAMPLE OF DESIGN RAINFALL GRID (12-HOUR, 1% AEP RAINFALL) FOR M09

3.2.2 Design Rainfall Temporal Patterns

Design temporal patterns (rainfall hyetographs) were provided by NRC for design modelling. These were developed as part of a previous project undertaken by Macky & Shamseldin (2020)[2]. The project aimed to provide multiple design hyetographs and a better representation of rainfall variability across the Northland region, replacing the single set of design hyetographs previously developed.

The HIRDS design temporal pattern is recommended for design modelling of Northland catchments[2]. Hence, the design hyetographs for the rainfall gauges were developed using the rainfall IFD data at available rainfall gauges for the catchment. Although a 12-hour hyetograph is suitable for design modelling for most Northland catchments as suggested[2], a range of durations were selected; including 1-hour, 6-hour, 12-hour and 24-hour for each design event, including 10%, 2% and 1% AEP events to ensure that the event critical duration was identified across the catchment. The shorter durations were critical in the upper parts of the catchment, while the longer 24-hour durations were critical in the lower catchment, where flood volumes are generally the predominant factor in generating peak flood levels.

Table 3-2 summarises the 1% AEP rainfall depth (based on IFD from HIRDSV4) for different event durations at each rainfall gauge and Figure 3-2 shows the design rainfall temporal patterns across different gauges for the 12-hour duration event. Considering a single temporal pattern is assigned (i.e. HIRDS hyetograph), the proportional amount of rainfall applied through time for a given duration (e.g., 6-hour) is generally consistent (as shown in Figure 3-2) across the catchment area.

[2] Macky & Shamseldin (2020) - Northland Region-wide Hyetograph review

TABLE 3-2 1% AEP DESIGN RAINFALL DEPTH

Gauge location	1% AEP (mm)			
	1-hour	6-hour	12-hour	24-hour
Fairburn_A53133	61	147	194	245
Honeymoon_valley_A53151	63	151	197	245
Kaingaroa_north_A53031	61	132	167	203
Kaitaia_aero_ews_A53026	63	140	176	213
MANGONUI_A43951	63	139	178	219
Oruaiti2_A53062	62	142	187	238
ORUAITI_A53061	63	160	217	286
RANGIPUTA_A43931	61	127	160	194
Rangitihi_A53131	62	140	181	225

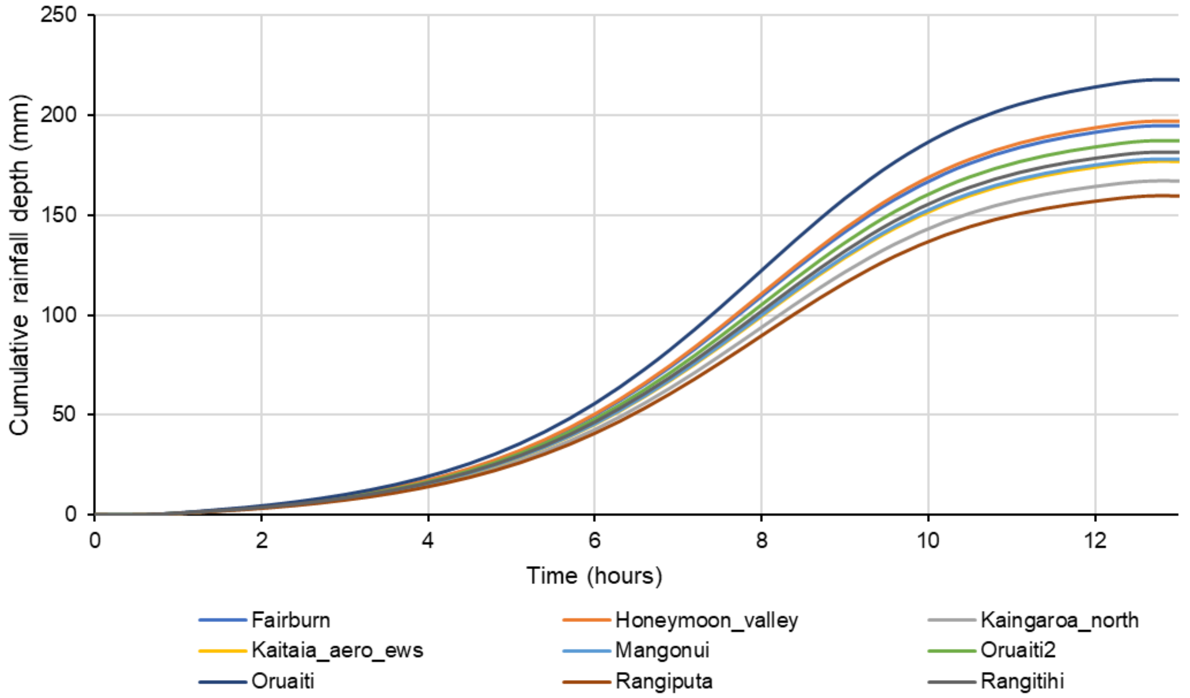

FIGURE 3-2 TEMPORAL PATTERN FOR DESIGN RAINFALL OF 12-HOUR, 1% AEP EVENT

A climate change scenario (for the 1% AEP events) was modelled for the 2081-2100 timeframe, for the RCP 8.5.

3.2.3 Losses

A series of land use types and importantly hydrological areas, were assigned a Manning's "*n*" (surface roughness), initial loss and a continuing loss. Table 3-3 summarises the adopted roughness and loss parameters. It should be noted these parameters were adopted based on the calibration to a historic event where streamflow gauges were present in other Far North catchments (i.e. M03, M06 and M07). Figure 3-3 displays the roughness layer based on the land use type, showing most land use is forest and grassland.

TABLE 3-3 DESIGN MODEL PARAMETERS

Hydrological areas	Land use types	Manning's n	Initial loss (IL) – mm	Continuing loss (CL) – mm/hr
Entire M09 catchment	Forest	0.09	9	6
	Grassland	0.05	9	4.5
	Cropland – perennial	0.04	17	2
	Cropland – annual	0.04	17	2
	Wetland – open water	0.04	0	0
	Wetland – vegetated	0.05	10	1
	Urban areas	0.10	5	1.5
	Waterways	0.05	0	0
	Other	0.06	15	1.5

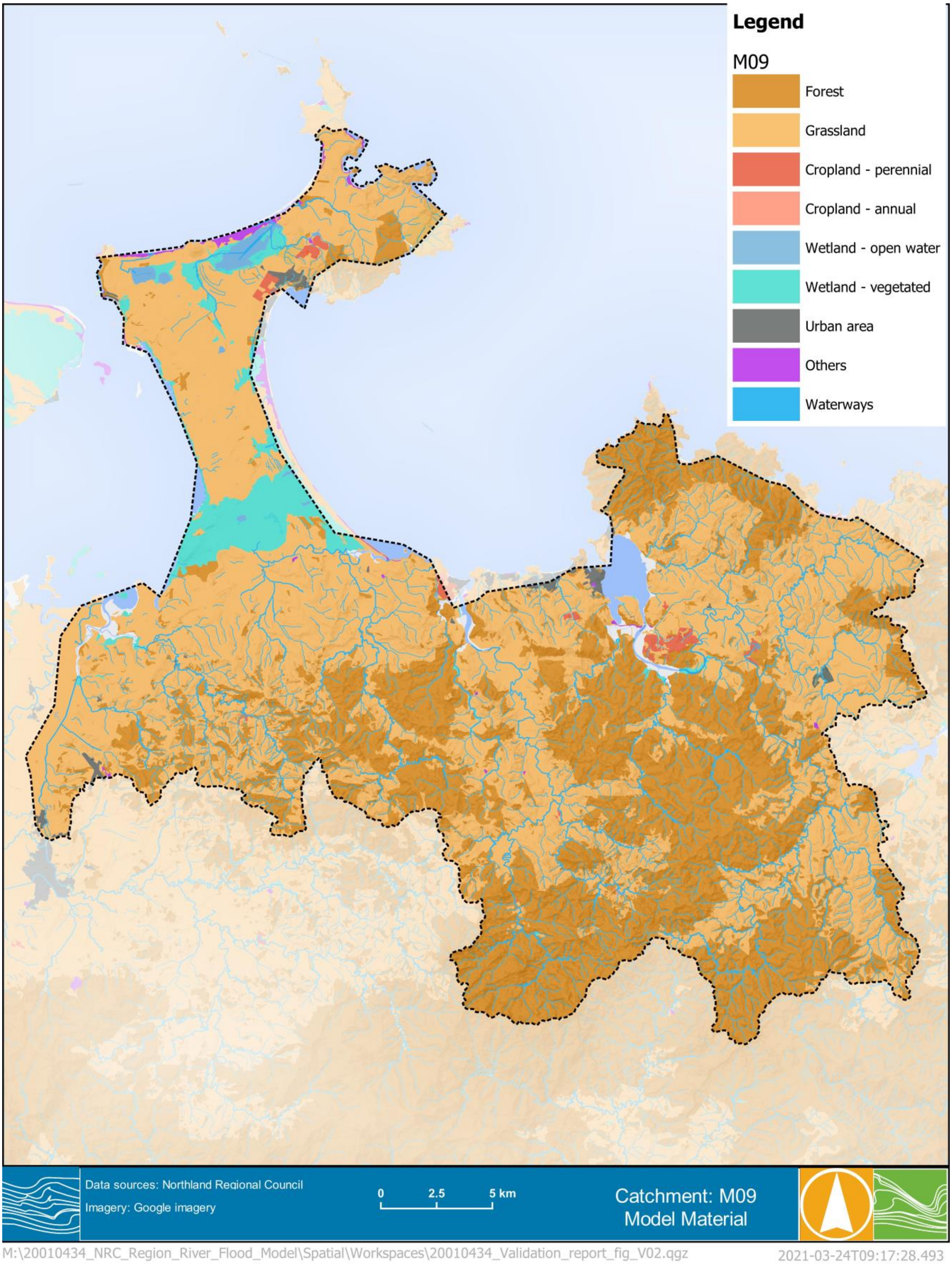

FIGURE 3-3 HYDRAULIC MODEL MATERIAL LAYER

3.2.4 Boundaries

As the Oruru catchment is a coastal catchment, a static tail-water (i.e. 1295 mm OTP) outflow boundary based on the 2 year ARI tide level[3] at Veronica Channel gauge was used for the design modelling. And a 1.2 m sea level rise was adopted for climate change runs.

There is an upstream inflow at the Pairatahi River coming from Awanui catchment (refer to Validation report for Awanui catchment) applied in this catchment model.

[3] MWH, 2010 *Priority Rivers – Flow Assessment, Sea Level Rise and Storm Surge*, prepared for Norhland Regional Council

4 MODELLING RESULTS

4.1 Modelled Result Processing/Filtering

Design modelling consisted of running the model for four storm durations (1-hour, 6-hour, 12-hour and 24-hour) with the results enveloped for each design event (i.e. 1%, 2% and 10% AEP) to ensure the critical duration was well represented across each part of the catchment. Each model run produced gridded results, including depth, water surface elevation (WSE), hazard (Z0) and velocity. Several post-processing steps were required to produce the final design modelling outputs. These are described as follows:

Step 1:

- The modelling results are firstly merged to produce the maximum outputs of the range of storm durations modelled. For example, the maximum 1% AEP flood depth is produced by merging the depth results of 4 different duration runs so the depth produced by the critical storm duration across each part of the catchment is well represented. Effectively, a map of the worst-case scenario at each location (based on the modelled scenarios) is generated across the whole area.

Step 2:

- The maximum gridded results are then remapped to a finer DEM grid using the 5-m LiDAR data. This allows the flood extent to be more accurately displayed on the map and the higher resolution gridded results (i.e. same resolution as the 5-m DEM) to be produced.

Step 3:

- Finally, the remapped results are post-processed by filtering out depths below 100mm and puddle areas less than 2000m^2 as agreed with NRC.

Figure 4-1, Figure 4-2 and Figure 4-3 respectively show the final post-processed flood depths, velocity and hazard of the 1% AEP design event modelled for M13. Figure 4-4 shows the flood depth map zoomed in at a township as an example. It is noted that the hazard classification is based on the following criteria:

TABLE 4-1 FLOOD HAZARD CLASSIFICATION

Hazard classification	Hazard – VxD (m^2/s)
Low	< 0.2
Low to Moderate	0.2 to 0.4
Moderate	0.4 to 0.6
Moderate to High	0.6 to 0.84
High	> 0.84

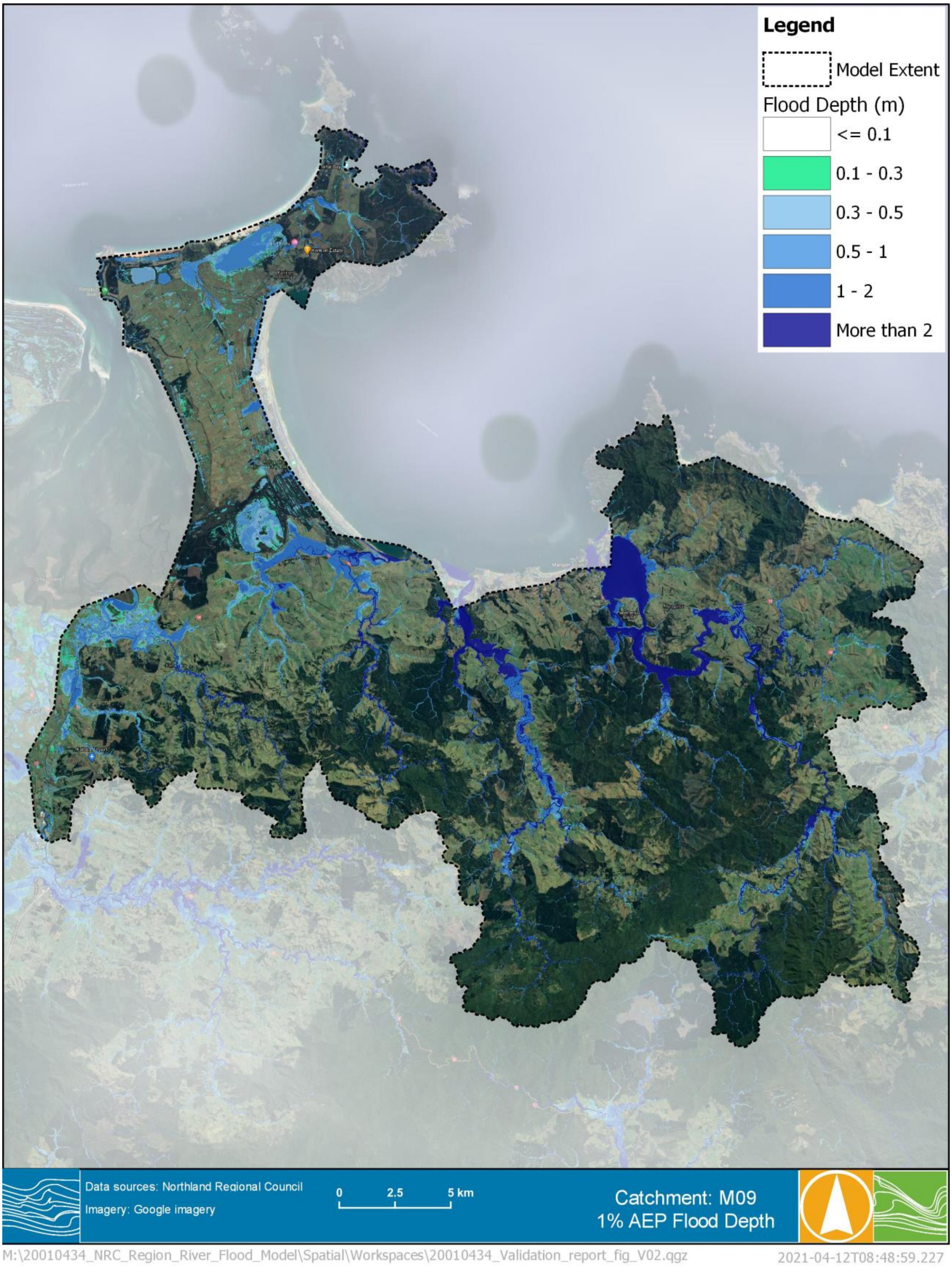

FIGURE 4-1 DESIGN MODELLING OF 1% FLOOD DEPTH

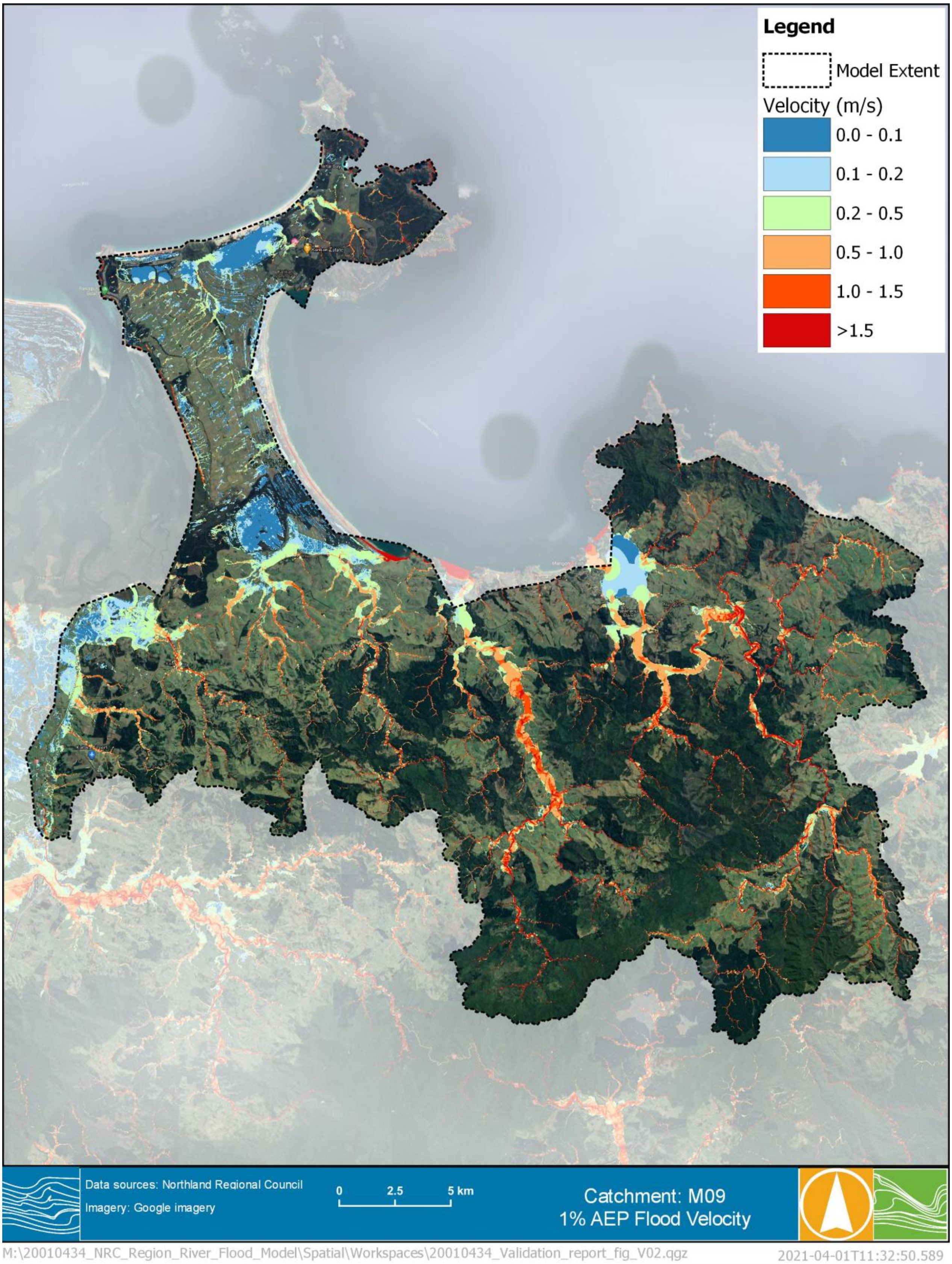

FIGURE 4-2 DESIGN MODELLING OF 1% AEP FLOOD VELOCITY

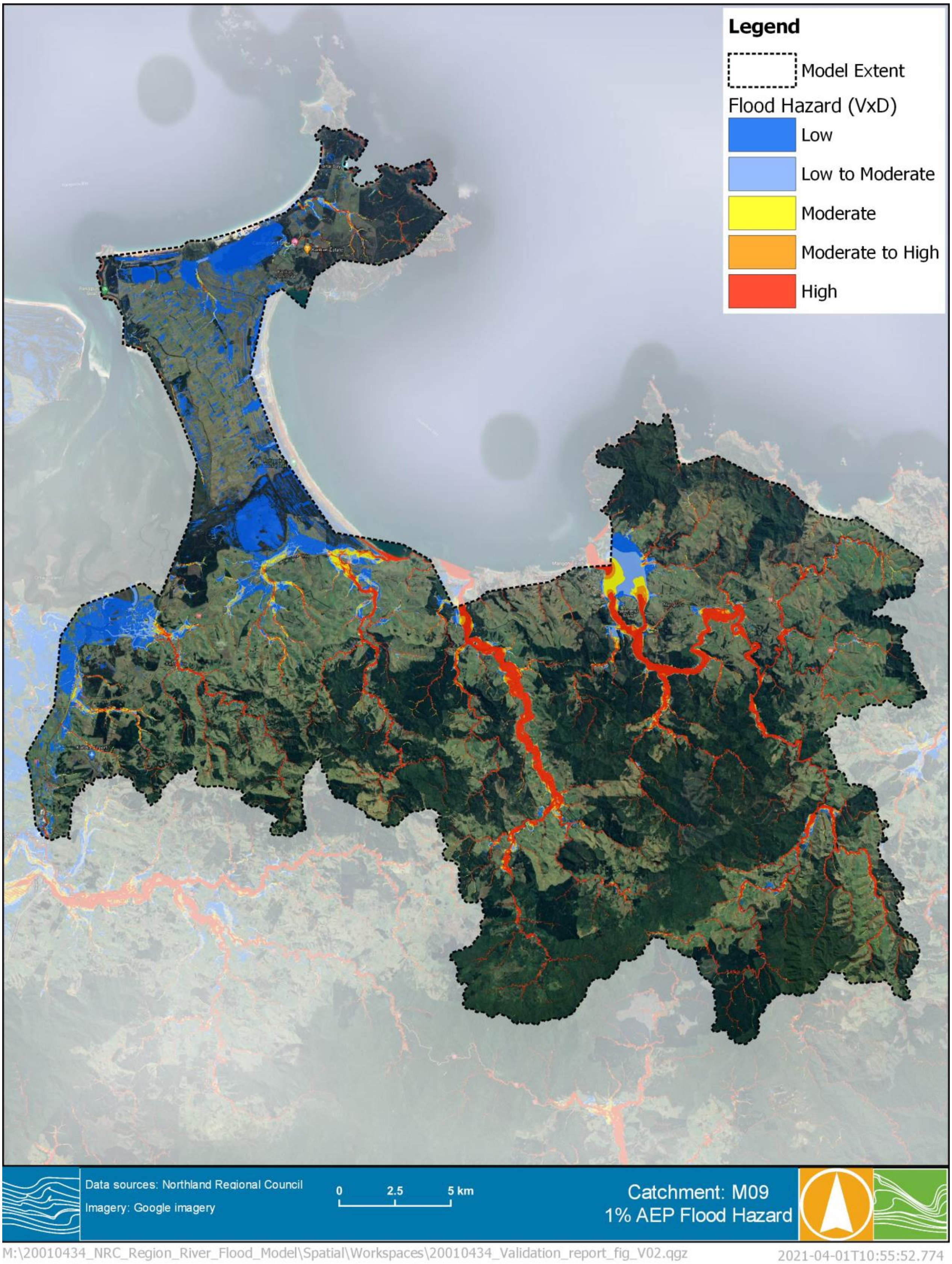

FIGURE 4-3 DESIGN MODELLING OF 1% AEP FLOOD HAZARD

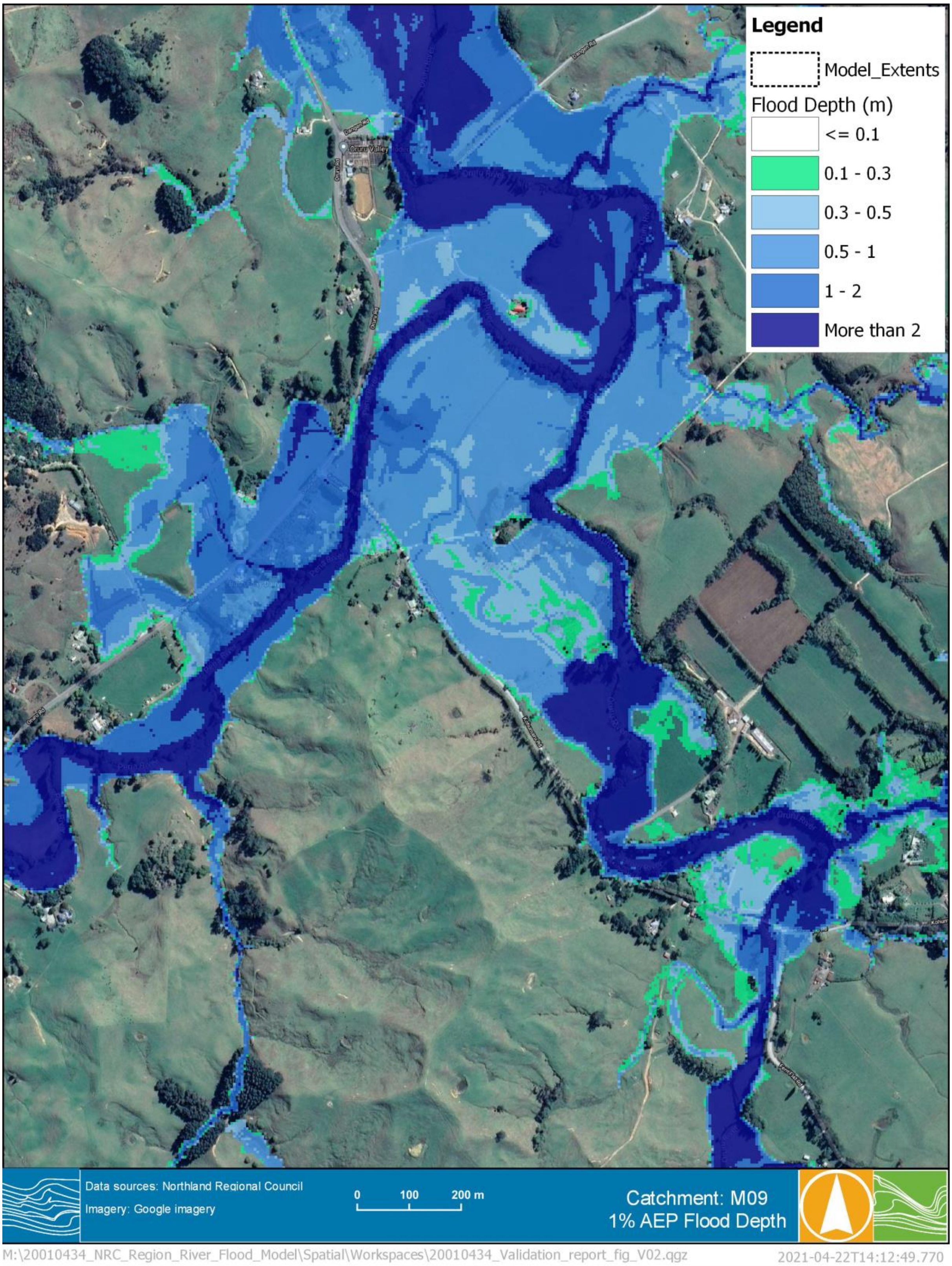

FIGURE 4-4 DESIGN MODELLING OF 1% AEP FLOOD DEPTH ZOOMED AT A TOWNSHIP

5 VERIFICATION OF DESIGN FLOWS

Flow lines were included at waterways and streamflow gauge in the hydraulic model as 2D Plot Output (2D PO) for design events. This allows flow hydrographs and peak flows to be extracted at these locations. Figure 5-1 displays the available streamflow gauge found within the Oruru catchment.

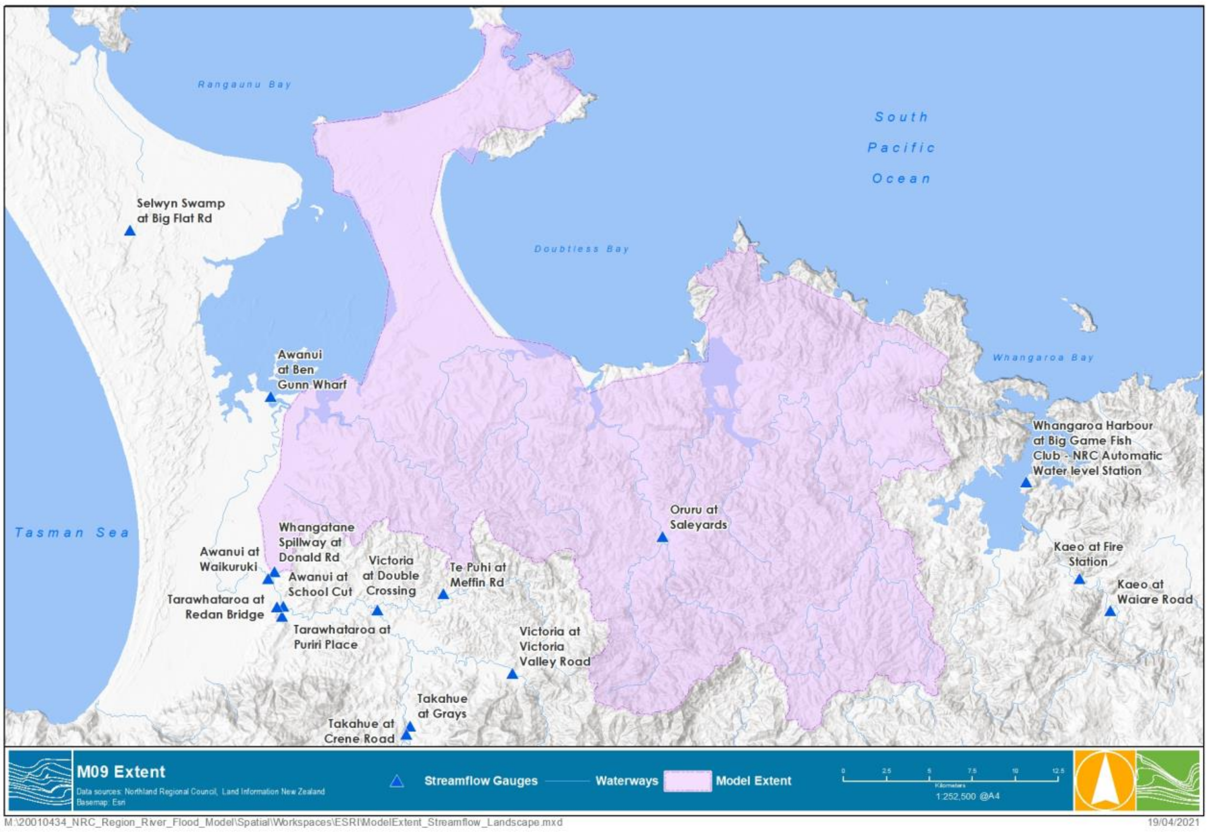

FIGURE 5-1 AVAILABLE STREAMFLOW GAUGES WITHIN ORURU CATCHMENT

The modelled peak flow for the 1% AEP design flood was compared with hydrological estimates, including FFA, rational method and SCS method, as well as observations from 2011 and historic maxima from streamflow gauge records.

5.1 Flood Frequency Analysis

A Flood Frequency Analysis (FFA) was undertaken for streamflow gauging stations with at least 25 years of records. The length of record for each gauge location was assessed to check if it met the 25+ year criteria as this can affect the reliability of the FFA especially for the estimation of major flood events (e.g. 1% AEP). The design flow estimates provide additional verification against the design hydraulic modelling results. The streamflow gauging stations that were selected for FFA and the corresponding 1% AEP flow estimates can be found in the Calibration Report (R01).

The annual series (maximum streamflow values for each year of gauge record) were calculated and input into FLIKE. FLIKE is a software package used for FFA and provides five different probability distributions for fitting the historical records. Log Pearson III distribution is commonly used across New Zealand and South-East

Australia to fit streamflow records and was used for all gauges within the study area. The FFA results have shown that this probability distribution has a relatively good fit in all the stations.

An example of the flood frequency curve by fitting the annual maximum streamflow values with the Log Pearson III distribution is shown in Figure 5-2. The design curve generated by the probability distribution shows a good fit with the historic records in more frequent events (i.e. 1 in 10 year or more frequent) but may slightly overestimate the design flows for rare events (e.g. 1% AEP flow). The flattening of the historic points may also suggest limitations with the current rating curves. Overall, the design curve shows a good fit with the tight confidence intervals indicating low uncertainty within these estimates.

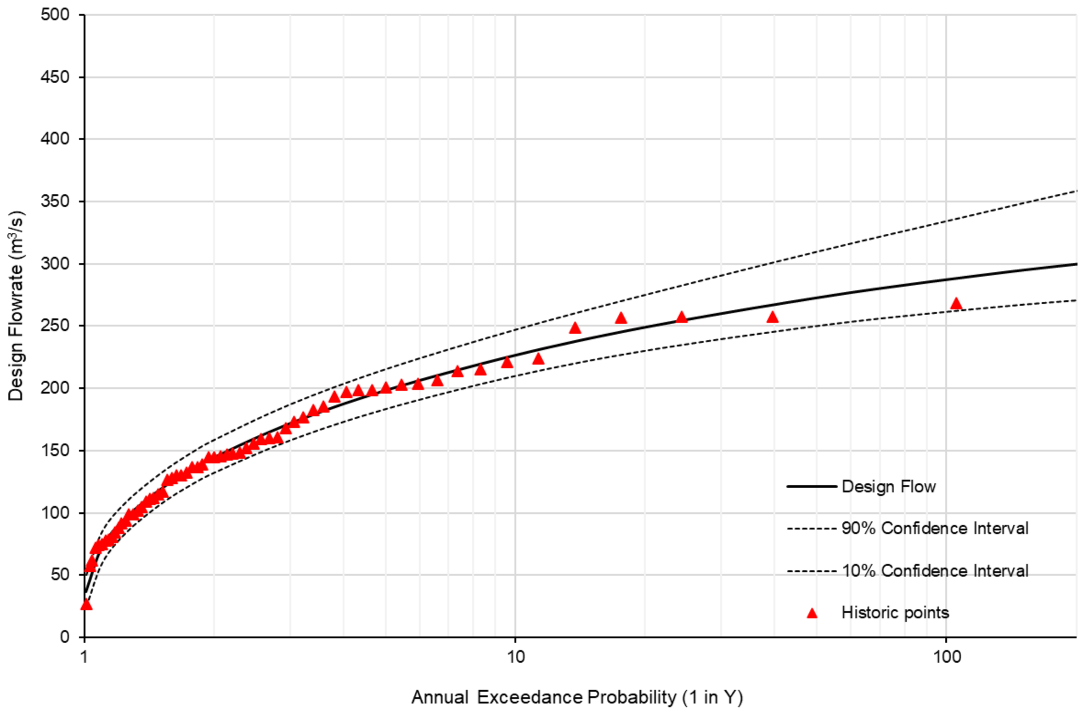

FIGURE 5-2 EXAMPLE OF FLOOD FREQUENCY CURVE OF LOG PEARSON III DISTRIBUTION FIT

5.2 Regional Estimation Methods

For catchments where a suitable streamflow gauge record was not available, additional estimation methods were used to provide design flow verification. These methods are based on empirical estimations using catchment area and design rainfall totals to verify design flows. These methods were checked for each streamflow gauge location within the study area and are described below.

5.2.1 NIWA New Zealand River Flood Statistics Portal

The New Zealand River Flood Statistics portal[4] provides peak flood estimation at streamflow gauging stations and the entire river system in New Zealand completed in 2018. The design estimates can be extracted from the portal are:

- Flood Frequency estimates (at flow gauge).
- Flood Frequency estimates, noted as Henderson & Collins 2018 (at river reach).

[4] NIWA Flood Frequency tool, accessed via: https://niwa.co.nz/natural-hazards/hazards/floods

- Rational Method HIRDS V3 (at river reach).

The flood frequency estimates given by the portal are determined using the Mean Annual Flow method developed by Henderson & Collins (2018)[5].

5.2.2 SCS method

The Soil Conservation Service (SCS) method, first developed by the U.S. Department of Agriculture's Soil Conservation Service, calculates peak flood flow based on rainfall and land-cover-related parameters. It is the recommended method for stormwater design in the Auckland region, providing a useful comparison. The peak flow equation is:

$$Q = (P - Ia)^2 / (P - Ia + S)$$

where:

- Q is run-off depth (millimetres)..
- P is rainfall depth (millimetres)
- S is the potential maximum retention after run-off begins (millimetres).
- Ia is initial abstraction (millimetres), which is 5 millimetres for permeable areas and zero otherwise.

The retention parameter S (measured in millimetres) is related to catchment characteristics through:

$$S = (1000/CN - 10)\ 25.4.$$

The value of the curve number (CN) represents the run-off from 0 (no run-off) to 100 (full run-off) and it is influenced by soil group and land use. A CN value of 50 was used for the SCS estimation of this catchment.

The run-off depth (Q) is then converted to a peak flow rate using the SCS unit hydrograph.

5.2.3 Rational Method

The Rational Method is widely used across both New Zealand and Australia. The equation is based on catchment area and design rainfall. The equation is:

$$Q = C\ i\ A / 3.6$$

where:

- Q is the estimate of the peak design discharge in cubic meters per second
- C is the run-off coefficient
- i is rainfall intensity in mm/hr hour, for the time of concentration
- A is the catchment area in km^2.

[5]Henderson, R.D., Collins, D.B.G., Doyle, M., Watson, J. (2018) *Regional Flood Estimation Tool for New Zealand Final Report Part 2*. NIWA Client Report

5.3 Verification Results

Table 5-1 summarises the comparison of 1% AEP peak flow estimates with the modelled values at Oruru at Saleyards gauge in the Oruru catchment and the differences between the estimation methods and modelled results can be visualised in Figure 5-3.

The rational method and the SCS method are only applicable for relatively small catchments, with the SCS method limited to 12 km^2. The catchment size for Oruru at Saleyards gauge is about 80 km^2. These equations are therefore subject to great uncertainty in summarising catchment characteristics.

At the Oruru at Saleyards gauge, the modelled design flow has a good match to the SCS method and the NIWA H&C 2018 estimate. This gauge only has 32 years of records available, making estimates relied on historic record such as FFA less reliable. Overall, the modelled peak flow at the Saleyards gauge tend to sit within a reasonable range of design flow estimates.

The use of empirical method estimations provides an additional degree of verification for streamflow gauges with only 32 years of record. It is also noted that the calibration process (i.e. calibration on other Far North catchments, this catchment is not calibrated) identified uncertainty with the streamflow records for high flows. The uncertainty of high flow extrapolation at these gauges could result in further uncertainty of flow estimate methods that rely solely on streamflow gauge data.

TABLE 5-1 SUMMARY OF 1% AEP PEAK FLOW COMPARISON

PO line location	Hydraulic model (m^3/s)		Records at gauge (m^3/s)		Empirical estimates (m^3/s)			NIWA Flood Frequency Tool 2018 (m^3/s)		
	Critical duration	Modelled peak	Jan 2011 peak	Highest on record	FFA	SCS	Rational method	NIWA – FF at gauge	NIWA – Rational method	NIWA – H&C 2018
Oruru at Saleyards	6 hr	282.8	101.1	101.1	111.7	218.3	165.2	126	N/A	263

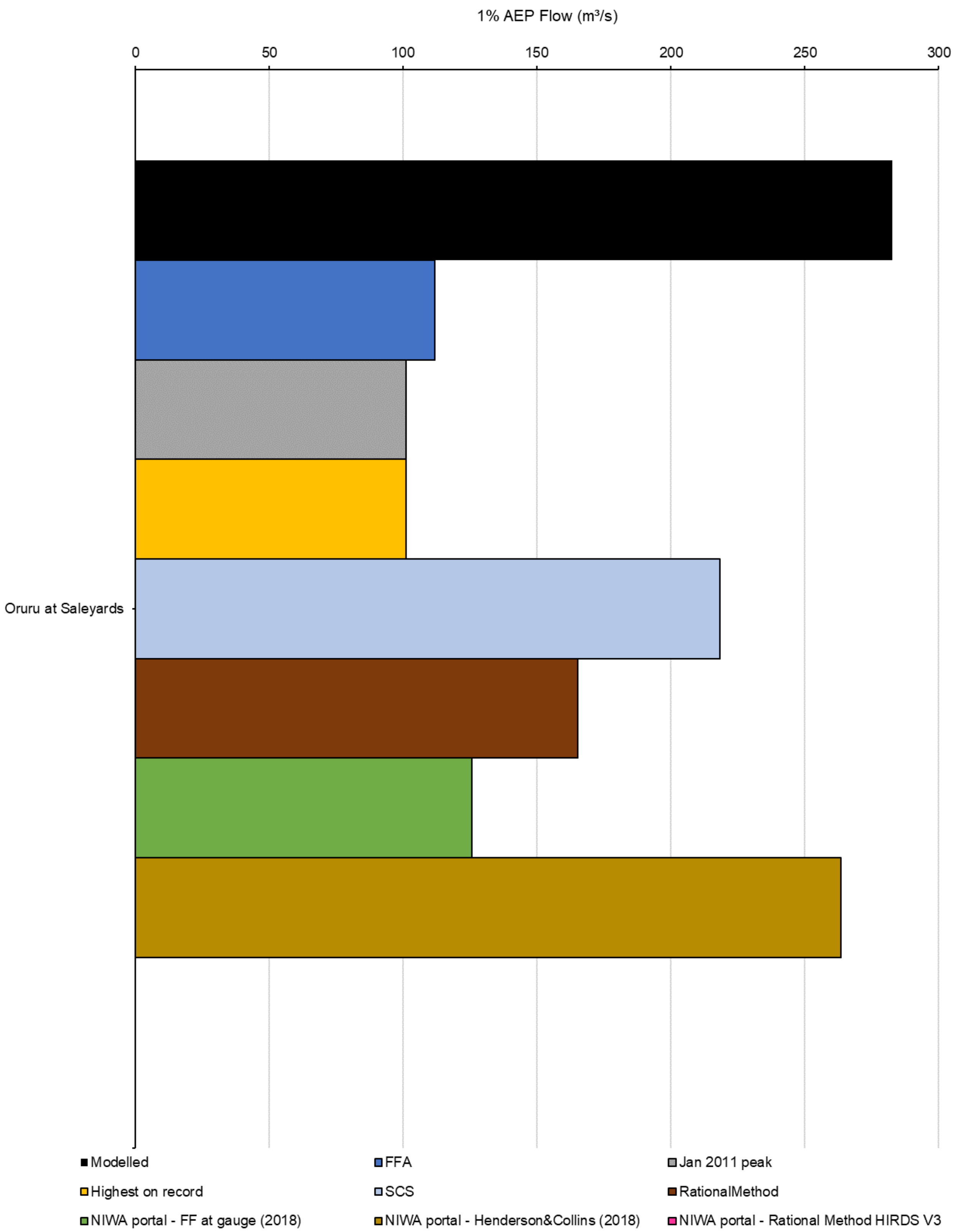

FIGURE 5-3 VERIFICATION OF DESIGN MODELLING RESULTS AGAINST HYDROLOGICAL ESTIMATES

6 SUMMARY

The Oruru catchment model (M09) was not calibrated and its model parameters were adopted based on calibrated catchments in the Far North region. The design modelling of this catchment consisted of four storm durations (1-hour, 6-hour, 12-hour and 24-hour) for each design AEP (i.e. 1%, 2% and 10% AEP). Design flood extents and gridded results, including depth, water surface elevation, velocity and hazard were produced and delivered to NRC.

The modelled 1% AEP design flows were verified against several design flood estimation methods at the streamflow gauging station. The comparison of design flows provides a general validation check of the modelled results given the accuracy of these estimation methods can be constrained by the reliability/availability of gauged flow records (where used, length of records) and general limitations with empirical design estimates. Overall, the modelled design flows at Oruru Saleyards streamflow gauge within the study area provide a reasonable fit to design flow estimates.

When considering the scope and the scale of this project, the current modelling results are considered fit for use. Modelling outputs can be used to identify flood hazard and potential flood risk. It can also inform planning decisions, infill flood mapping between detailed flood studies and provide a basis for broad emergency management exercises.