

Appendix 20

Stormwater Assessment



STORMWATER POND ASSESSMENT REPORT FOR THE PROPOSED NORTHPOR EXPANSION

PREPARED FOR NORTHPOR
AT PORT MARSDEN HIGHWAY, MARSDEN POINT



NORTHPORT STORMWATER POND ASSESSMENT REPORT

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Appendix A – HydroCAD summary reports

Appendix B – Consent CON20090505532 – Schedule 1

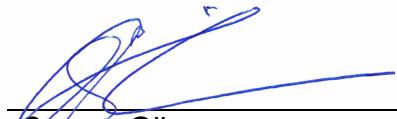
Appendix C – Survey Plan

Appendix D – Northport Stormwater Pond Plan and Detail

Appendix E – Pond Levels Schematic

Appendix F – Figure 01 – Overall Site Plan

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1. Purpose

The purpose of this report is to assess the current Northport stormwater settlement treatment pond system, in terms of treatment, volumetric capacity and compliance with the existing stormwater resource consent, for both the existing port catchment and the proposed future port expansion. Recommendations to address stormwater runoff from the proposed expansion areas are also provided.

This report is intended to inform a resource consent application with the Northland Regional Council (NRC) for the expansion of the port facility.

2. Proposal

It is proposed to expand the port apron from the current 49Ha footprint to a total 67.3Ha. This includes an already consented expansion area of 4.6Ha, and a further expansion of 13.7Ha (being the current proposal). These expansions will result in an increase in stormwater runoff requiring treatment prior to discharge.



Figure A: Proposed Northport expansions

An assessment of the existing stormwater system has been undertaken to establish whether compliance with the existing resource consent (refer section below) can be maintained and verify whether adequate capacity is available within the system to provide the required treatment whilst accommodating the additional runoff from the proposed port expansions.

3. Existing Consent Conditions

A consent is held by Northport to discharge stormwater associated with the operation of the port to the Whangarei harbour, after treatment via the storage and settlement pond system – CON20090505532 (refer to Appendix B).

The discharge consent conditions are limited to water quality and treatment rather than a discharge volume or rate. However, there is an advice note under section 1.2 Pumping Hours within Schedule 1 of the application which states that '*the approximate average volume of stormwater to be discharged is assessed at 200,000 cubic metres per annum. The size of the discharge pipe and the proposed capacity of the pumps limit the pumped discharge rate to approximately 2,520 cubic metres per hour*'. It is clear that the referenced 200,000m³ per annum is incorrect, since this represents only 408 hours (17.0 days) of pumped discharge alone, at the maximum pump rate is 490m³/hr. It is recommended that the reference to total annual volume of discharge is removed noting that peak rates of discharge are principally limited by the pumping configuration and discharge pipe size.

4. Existing Northport Pond

The Northport stormwater settlement pond was constructed to provide treatment to stormwater runoff from the port prior to discharge to the Whangarei harbour. In 2016 the pond was extended to accommodate a first stage port expansion, and in 2018 baffles and two forebay bunds were installed within the pond to limit inflow "shortcutting". Refer to the Boundary Hunter Ltd survey plan for the pond layout – Appendix C.

A canal system along the perimeter of the port apron collates and directs runoff from the port to the stormwater pond. With an average depth of 1.4m and base width of 2.6m, the canal has the capacity to convey flows of up to approximately 3.8m³/s (ignoring any restrictions resulting from culverts). An inlet weir from the canal then discharges the flows into the pond via the forebays. A central bund, which has been constructed with gabion rock, splits the pond in two. Flows discharge both over and through the bund, around the baffles, and are then pumped from the second basin into a gravity pipeline to combine with discharges from another industrial land treatment pond immediately southeast of the subject pond (independent consent) before ultimately discharging to the Whangarei harbour via a diffuser at depth under Berth 1 of the berth face. Refer to Figure 01 for the overall site plan for the port and pond stormwater management - Appendix F.

The discharge from the second basin of the pond is via duty-assist pumping system. Records show the pump rates to be at 290m³/hr for the duty pump and at 490m³/hr when the standby pump is activated (i.e. both pumps running concurrently).

The gravity pipeline from the pump discharge is a 525dia which increases to a 825dia at the gravity inlet from the forebay area scruffy dome overflow, as indicated on the Northport drawing D60-30-06-01-004 dated Feb 2002 – Appendix D. The gravity line then increases further to a 1500dia at the point at which the 1200dia outlet pipe from the Marsden Maritime Holdings (MMH) quality management pond connects into the line. Refer to the Northport drawing D60-30-06-01-012 dated August 2002 – Appendix D.

There are two formalised overflows from the stormwater system. One is located in the first half of the pond, being a scruffy dome manhole riser approximately 520mm above the pond inlet weir level discharging directly to the 825dia gravity outlet pipeline bypassing the pump limitation, and the other is a 9m long spillway at the northern end of the western canal, at a level 300mm below the port apron level, which discharges directly to the harbour.

Monitoring results demonstrate that the pond currently meets the water quality requirements as detailed in Schedule 1 of the consent – CON20090505532 (Appendix B).

Water quality monitoring of the discharges (spills) from the western canal spillway have not been undertaken, although the western spillway only functions during larger events (see section 7) and discharges from the top of the canal system, thus retaining sediments in the treatment network and discharges here will be significantly after first flush flows. Furthermore, treatment is designed to be provided for higher frequency rainfall events (1/3 of 2-year ARI) which will be conveyed to the treatment pond.

5. Assessment – Treatment Capacity

The original treatment design for the port was based on Auckland TP10¹ (now replaced by GD01)². As the current pond is meeting the water quality requirements of the consent it is considered appropriate to continue this design basis. It should also be noted that the current consent conditions are more stringent than the thresholds set out in the proposed Regional Plan and thus confirming the conservativeness of the design.

In accordance with Auckland TP10 the required water quality volume (WQV) for the current port apron area of 49Ha is 13,480m³. This volume is required to be provided as dead storage within the treatment network.

Survey information indicates that the available dead storage in the first half of the pond, below the level of the central bund at 3.75m CD, is 12,320m³. The dead storage available in the second half of the pond, below the pump off level of 3.10m CD, is 5,430m³. Therefore, the total dead storage available within the existing pond is 17,750m³ providing more than the required WQV for the existing port apron areas.

The proposed full expanded port footprint of 67.3Ha will require a total WQV of 18,510m³ which exceeds the dead storage volume currently available in the existing pond by 760m³. However, this additional dead storage can be provided within the base of the conveyance canals on the site at a depth of approximately 110mm (assuming a total canal length of 2640m – canals to full expanded port perimeter).

¹ Stormwater Management Devices: Design Guidance Manual.

² Stormwater Management Devices in the Auckland Region.

During rainfall events up to the treatment threshold (1/3 of the 2-year – 24hour rainfall), the effectiveness of a 110mm depth of dead storage to treat flows would likely be compromised, with the increase in hydraulic gradient and the resulting velocities. Therefore, the recommendation is to install a 250mm high weir (set at 3.85m CD) within the canal which will provide an additional 2010m³ of dead storage, mitigating the effects of the increased velocities in the canal.

With the installation of the weir a total dead storage volume of 19,760m³ will be provided, 1250m³ more than required, continuing the conservative treatment design approach. This will provide a staged WQV, where removal of coarse sediments and debris is addressed within the canals, finer treatment within the first basin of the pond, and polishing within the second pond basin (pump pond).

6. Model Build and Calibration

To assess the volumetric capacity of the pond for the existing port apron and the proposed future expansion, a model of the catchment, inflows and pond has been built in the HydroCAD hydrologic modelling software (refer to Appendix A for HydroCAD summary reports). This model has been calibrated against a historic rainfall event of the 24 December 2018, using available pond and canal level data and pump operating data records to establish the magnitude of initial losses and exfiltration.

The storage volumes within the pond have been established from survey information provided by Boundary Hunter Ltd, dated February 2021. Storage volumes within the canal have been estimated from survey sections of the canal and canal lengths measured from GoogleEarth aerials.

It is noted from GoogleEarth aerials that the calibration event (December 2018) is prior to the installation of a 1200dia culvert within the western canal and during this period a portion of the southern apron was under construction with sediment ponds in place. Refer to the figure below. This has been reflected in the model calibration.

The pond pumps have been included in the model with operational stop/start levels as advised by Northport. Refer to the pond level schematic – Appendix E. Pump 1 is modelled as having a maximum pump rate of 290m³/hr and pump 1 and pump 2 having a combined maximum pump rate of 490m³/hr, as established from Northport pump data records.

The existing inlet weir, the central pond bund, and the scruffy dome overflow have all been built into the model with elevations, dimensions and lengths taken from survey information. The Inlet weir (outlet from the canal) being 10m in length and at an elevation of 3.95m CD and the central bund with a length of approximately 75m and at 3.75m CD. The scruffy dome manhole riser has a 2100dia and the rim level is set at 4.47m CD.

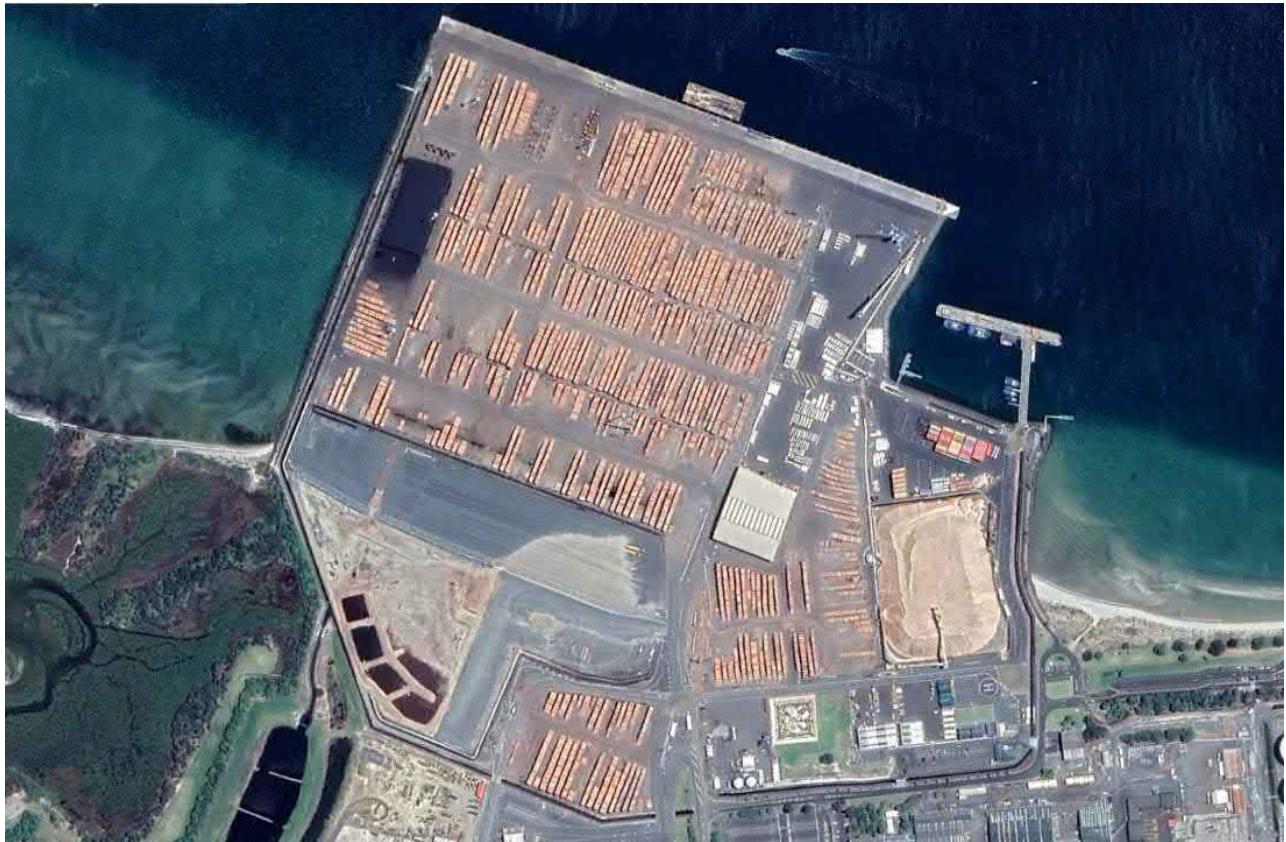


Figure B: GoogleEarth aerial Dec 2018

6.1. Calibration Event

The selection of the rainfall event to be used for calibration of the pond model was based on a review of pond and canal level data provided, with quality data being available for the selected event. The calibration rainfall event was selected from the period between the installation of the forebays and pond baffles in 2018 and prior to mid-2019. The rainfall event being that which occurred on the 24 December 2018, which, while “significant”, is not extreme and therefore is unlikely to distort pond behaviour.

Rainfall data was obtained from the NRC – Marsden Point rainfall gauge for 24 December 2018 which showed a total rainfall depth for the event of 80mm over a 24hour period (equivalent to a 2-year ARI rainfall event) and a peak intensity of 40.6mm/hr for 10 minutes (equivalent to that of a 10-year ARI rainfall event).

Calibration of the pond model for the selected event, against level data, indicated noticeable exfiltration from the pond and canal, and that discharge through the central pond bund occurs. An exfiltration rate of 20mm/hr from the pond and canal, above the static water elevation, was incorporated in the model (unlined pond in sand soils). An estimated 50l/s was allowed for flows through the pond bund into the second basin (estimated from alignment of model behaviour verses data).

For the calibration event pump record data shows a total pumped discharge from the pond of 23,932m³. The modelled discharge from the pond is calculated as 23,993m³ being within 1% of the recorded data discharge volume. The modelled peak pond elevation is 3.860m, being 60mm lower than the recorded data. The accuracy of the calibration is considered suitable for the purposes of this model's use, noting variability is likely over a range of event depths and durations.

7. Assessment – Volumetric Capacity

7.1. Current Port

Utilising the calibrated model for the current port apron of 49Ha (with the addition of the 1200dia culvert in the western canal and the seal to the southern portion of the port apron completed), the pond performance has been assessed using a Type 1A nested rainfall hyetograph methodology and site-specific rainfall data obtained from HIRDS version 4.

The starting water elevations in both halves of the pond have been set at 3.30m CD and 3.10m CD, respectively. This represents the first basin at approximately half full and second basin at the pump off level. These levels are elevated from those of the recorded level data from the calibration event and are therefore considered conservative.

The model shows that the 1200dia culvert in the western canal creates a restriction on flows from the western catchment, resulting in the canal spillway operating during events in excess of a 10-year ARI event. However, the model also shows that the pond has the capacity to address the runoff generated by the current port apron area that reaches the pond (canal spillway operating) for events up to and including the 100-year ARI rainfall event and that the 825dia discharge pipe has adequate capacity to convey the peak discharge of 0.65m³/s from pumps and scruffy dome overflows. The peak pond elevation for the 100-year ARI event is shown to be at 4.59m CD being 410mm below the port apron level of 5.00m CD. For events exceeding the 20-year ARI event, the model indicates that the scruffy dome overflow will operate within the pond first basin.

For the 100-year ARI event the model shows the western canal spillway to operate with a peak discharge of 0.89m³/s and total discharge volume of 1560m³, being 1% of the total runoff volume from the expanded port apron. The modelled peak elevation in the western canal for the 100-year ARI event is 4.86m CD, being just 140mm below the port apron level.

The failure mode for the system, beyond that of the event limits discussed, has not been examined fully. However, flooding of the current port apron is unlikely for any event with a rainfall depth of less than 350mm over 24hrs (extreme). If the pond pumps were to fail the current system (scruffy dome and western canal spillway) would be able to address a 100-year ARI event from the current port apron, with peak pond elevation at 4.62m CD (380mm below the port apron).

7.2. Full Port Expansion

To accommodate the required treatment volume in the system for the full port expansion (as discussed in section 5), the starting water level in the canals has been set at 3.85m CD. An allowance for an additional 670m of canal has been included within the model for the 67.3Ha port catchment. The model indicates that the western canal spillway operates for events in excess of the 10-year ARI event and overflows from the pond will occur for events exceeding that of the 5-year ARI rainfall event, with the capacity of the 825dia outlet exceeded for events over that of a 20-year ARI event.

For the 100-year ARI event the model shows the western canal spillway to operate with a peak discharge of 0.89m³/s and total discharge volume of 7,710m³. The calculated peak elevation in the western canal for the 100-year ARI event is 4.860m CD; being just 140mm below the port apron level.

In summary, the assessment of the pond capacity identifies that the existing unmodified pond is confirmed to have the capacity to address the 100-year ARI event runoff received by the pond from the current port footprint (noting the operation of the western spillway reduces the flows conveyed to the pond), but the capacity to address the additional runoff from the full proposed port expansion is limited to that of the 20-year ARI event.

8. Recommendations

To provide the additional treatment volume required to address the port expansion it is recommendation that a 250mm weir is constructed in the canal system, directly upstream of the pond inlet weir.

To address the capacity requirements of the proposed expansion, the recommendation is for a second canal spillway to be installed to provide relief to the network. The recommended location would be near the eastern limit of the eastern canal, discharging to the harbour.

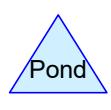
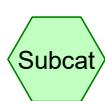
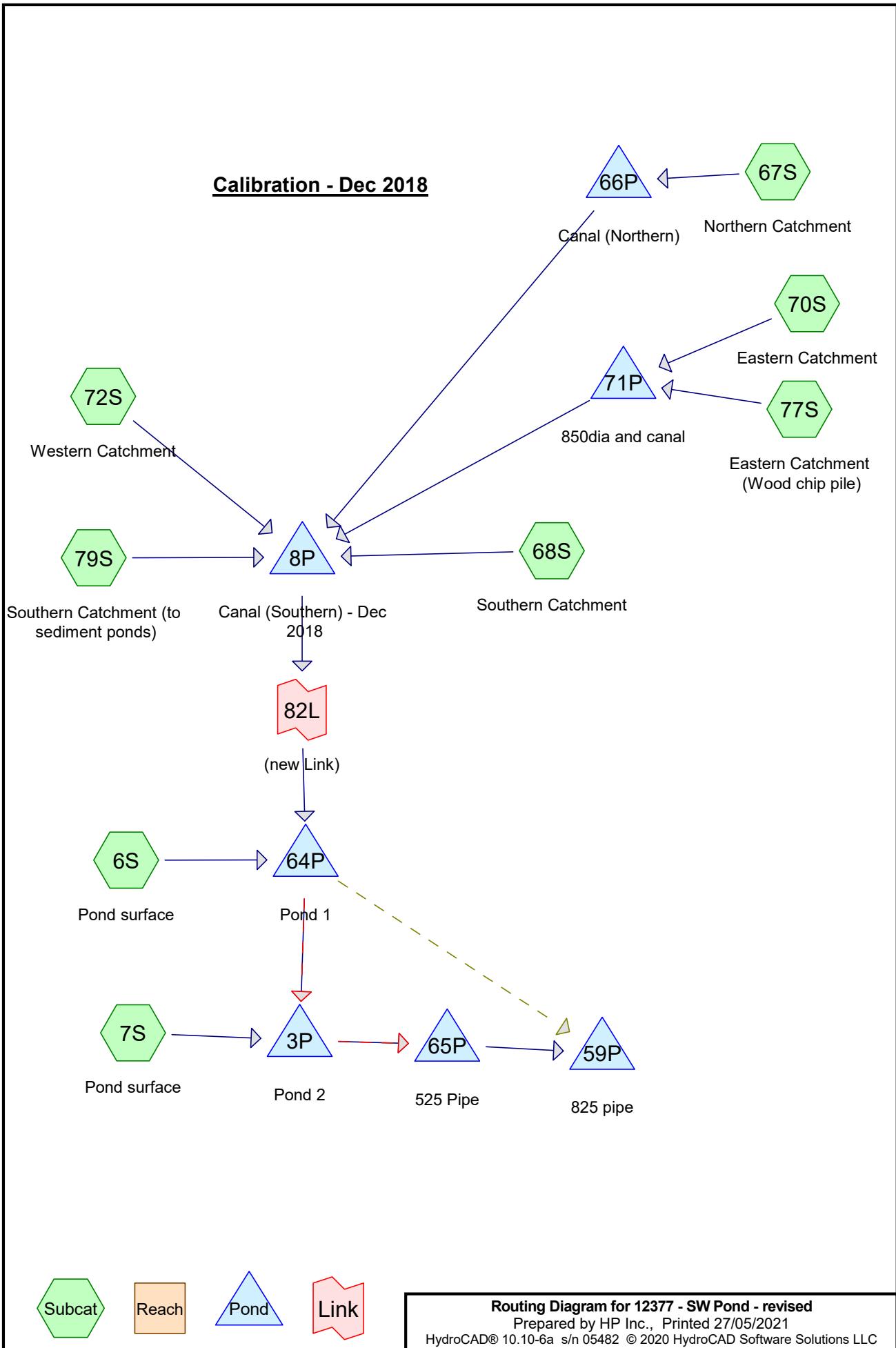
Calculations show that for the stormwater network (canals, pond, and overflows) to have the capacity to address the 100-year ARI rainfall event for the port expansion a 10m length of spillway will be required at 490mm below the port apron level. Based on using a Type 1A nested rainfall hyetograph methodology and rainfall data from HIRDS version 4, the spillway will start to operate for events in excess of the 10-year ARI event, with the 100-year ARI peak discharge calculated at 0.56m³/s and total discharge volume of 21,420m³. The 100-year ARI peak elevation in the canal is calculated at approximately 380mm below the port apron level.

It is recommended that the existing spillway (western) and any future spillway installed be modified/designed to retain oils and floatables within the port/canal network with an under-over weir setup, as presently the configuration permits loss of control of these aspects of a spill.

9. Limitation

This report has been prepared solely for the benefit of our client Northport Ltd and the Northland Regional Council in relation to the resource consent for which this report has been prepared. The comments in it are limited to the purpose stated in this report. No liability is accepted by Hawthorn Geddes engineers & architects Ltd in respect of its use by any other person, and any other person who relies upon any matter contained in this report does so entirely at their own risk.

Appendix A – HydroCAD summary reports



Routing Diagram for 12377 - SW Pond - revised

Prepared by HP Inc., Printed 27/05/2021

HydroCAD® 10.10-6a s/n 05482 © 2020 HydroCAD Software Solutions LLC

Summary for Subcatchment 6S: Pond surface

Runoff = 0.2637 m³/s @ 91.16 hrs, Volume= 2.024 MI, Depth= 92 mm
 Routed to Pond 64P : Pond 1

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (m ²)	CN	Adj	Description
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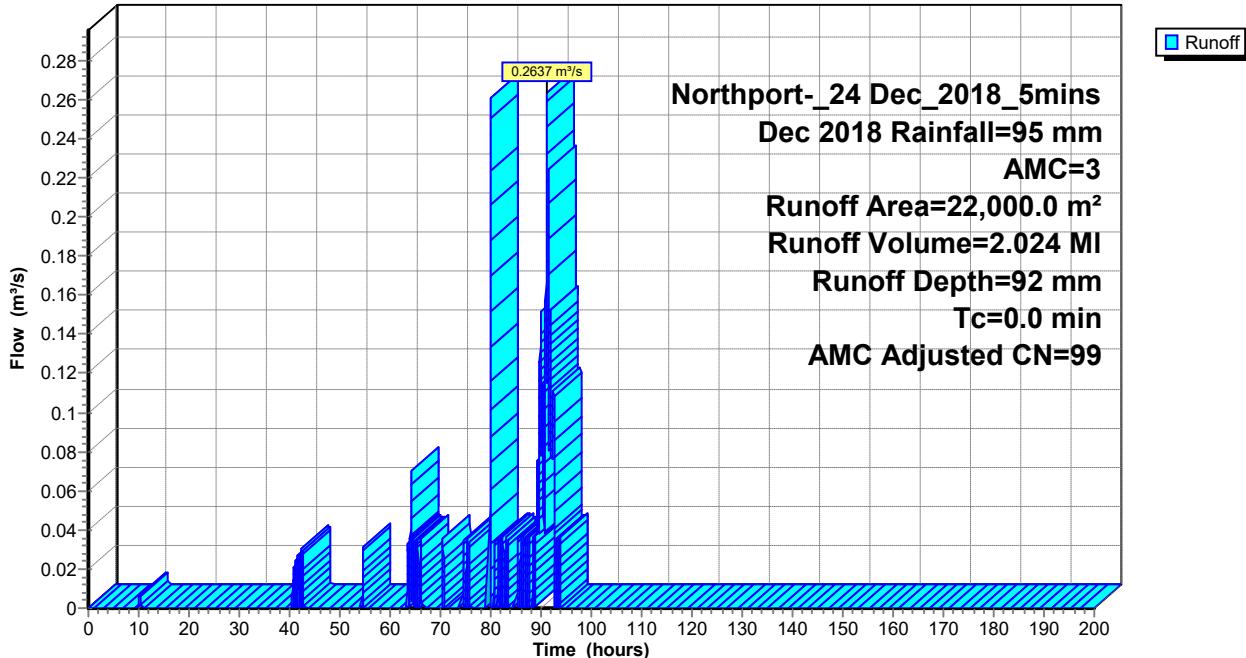
*	22,000.0	98	
	22,000.0	98	Weighted Average, AMC Adjusted
	22,000.0	99	100.00% Impervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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				0.0	Direct Entry,
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Subcatchment 6S: Pond surface

Hydrograph



Summary for Subcatchment 7S: Pond surface

Runoff = 0.4195 m³/s @ 91.16 hrs, Volume= 3.220 MI, Depth= 92 mm
 Routed to Pond 3P : Pond 2

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (m ²)	CN	Adj	Description
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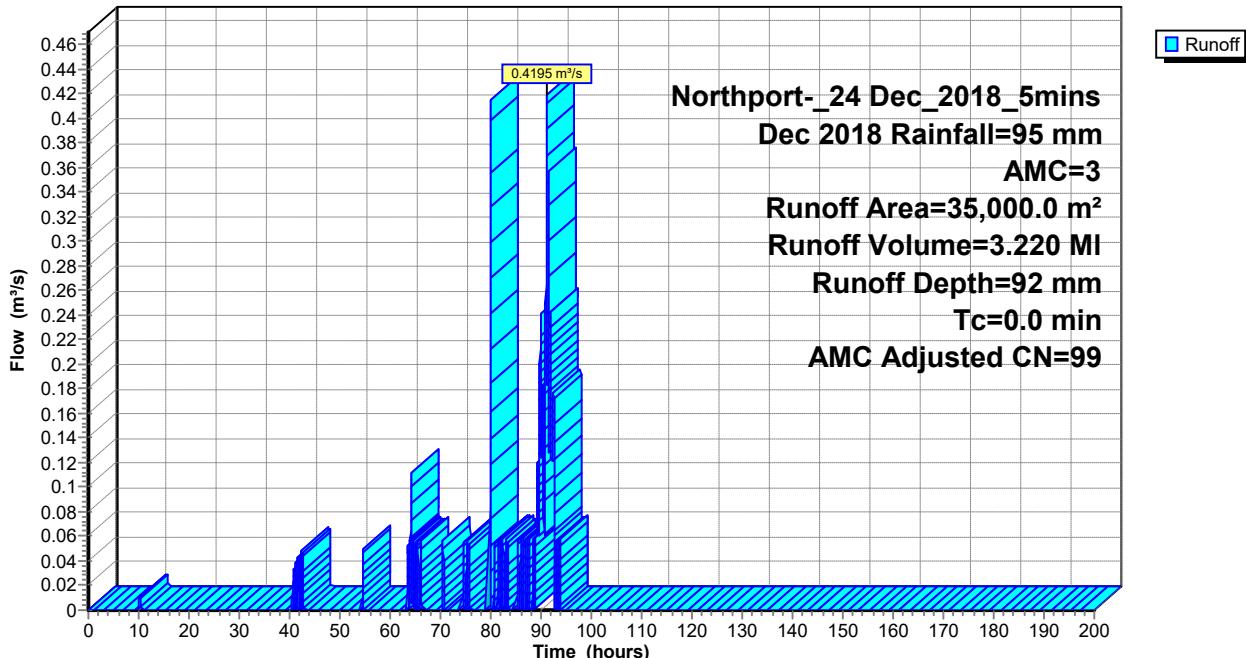
*	35,000.0	98	
	35,000.0	98	Weighted Average, AMC Adjusted
	35,000.0	99	100.00% Impervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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				0.0	Direct Entry,
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Subcatchment 7S: Pond surface

Hydrograph



Summary for Subcatchment 67S: Northern Catchment

Runoff = 0.5101 m³/s @ 91.56 hrs, Volume= 6.033 MI, Depth= 86 mm
 Routed to Pond 66P : Canal (Northern)

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (ha)	CN	Adj	Description
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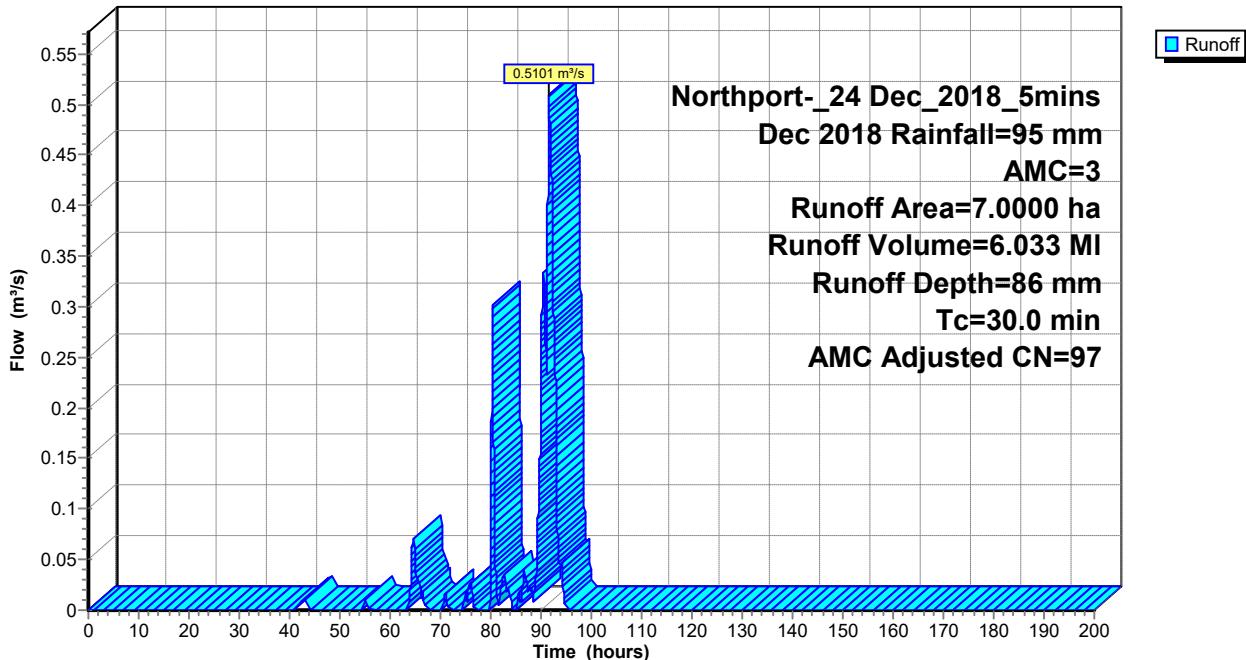
*	7.0000	92	
	7.0000	92	Weighted Average, AMC Adjusted
	7.0000		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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30.0					Direct Entry,
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Subcatchment 67S: Northern Catchment

Hydrograph



Summary for Subcatchment 68S: Southern Catchment

Runoff = 0.6247 m³/s @ 91.56 hrs, Volume= 7.213 MI, Depth= 83 mm
 Routed to Pond 8P : Canal (Southern) - Dec 2018

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (ha)	CN	Adj	Description
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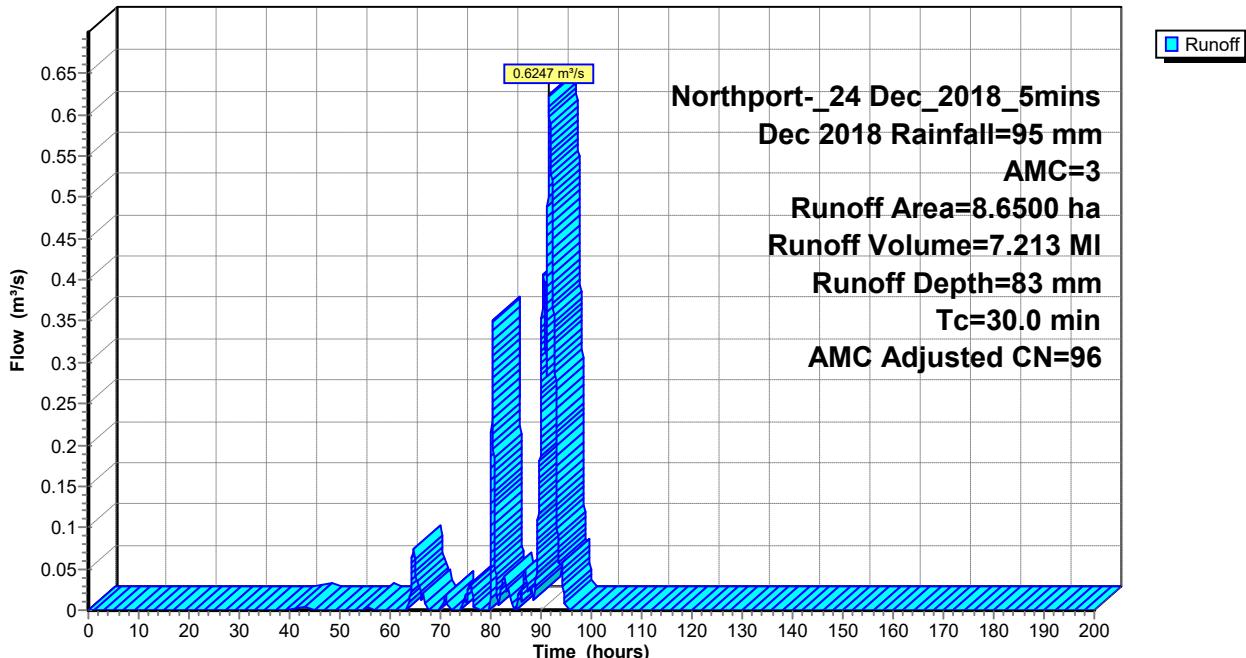
*	8.6500	90	
	8.6500	90	Weighted Average, AMC Adjusted
	8.6500	96	100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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30.0					Direct Entry,
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Subcatchment 68S: Southern Catchment

Hydrograph



Summary for Subcatchment 70S: Eastern Catchment

Runoff = 0.3535 m³/s @ 91.56 hrs, Volume= 4.180 MI, Depth= 86 mm
 Routed to Pond 71P : 850dia and canal

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (ha)	CN	Adj	Description
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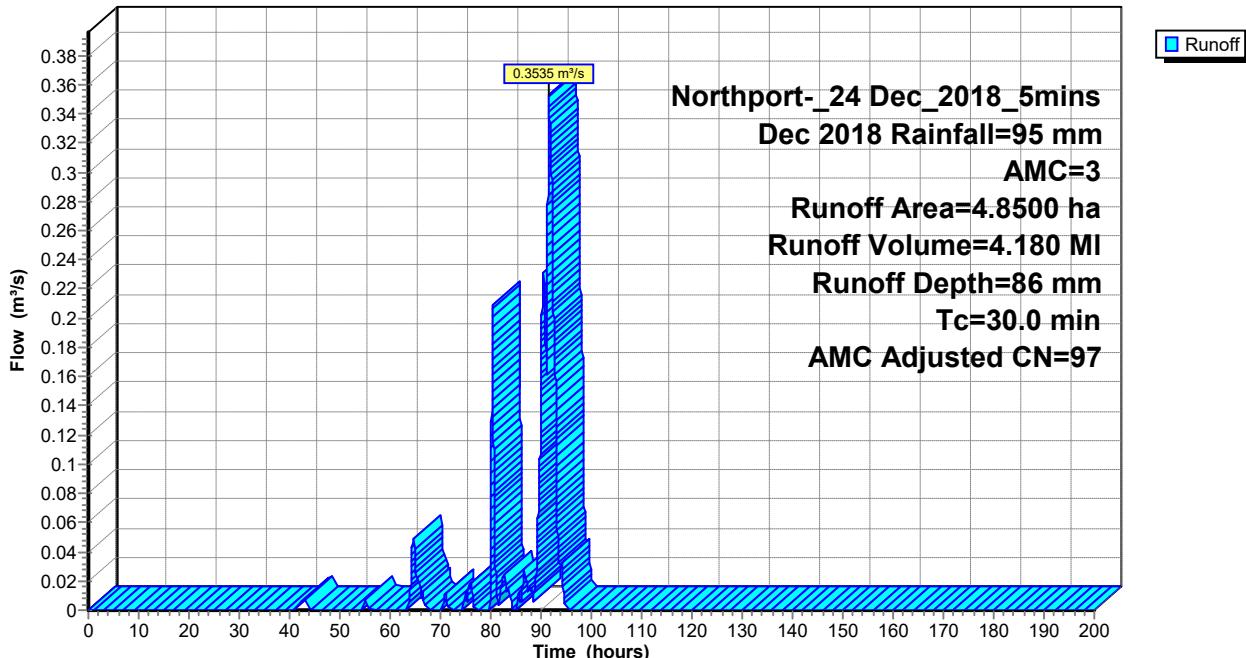
*	4.8500	92	
	4.8500	92	Weighted Average, AMC Adjusted
	4.8500		100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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30.0					Direct Entry,
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Subcatchment 70S: Eastern Catchment

Hydrograph



Summary for Subcatchment 72S: Western Catchment

Runoff = 1.5304 m³/s @ 91.56 hrs, Volume= 18.098 MI, Depth= 86 mm
 Routed to Pond 8P : Canal (Southern) - Dec 2018

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (ha)	CN	Adj	Description
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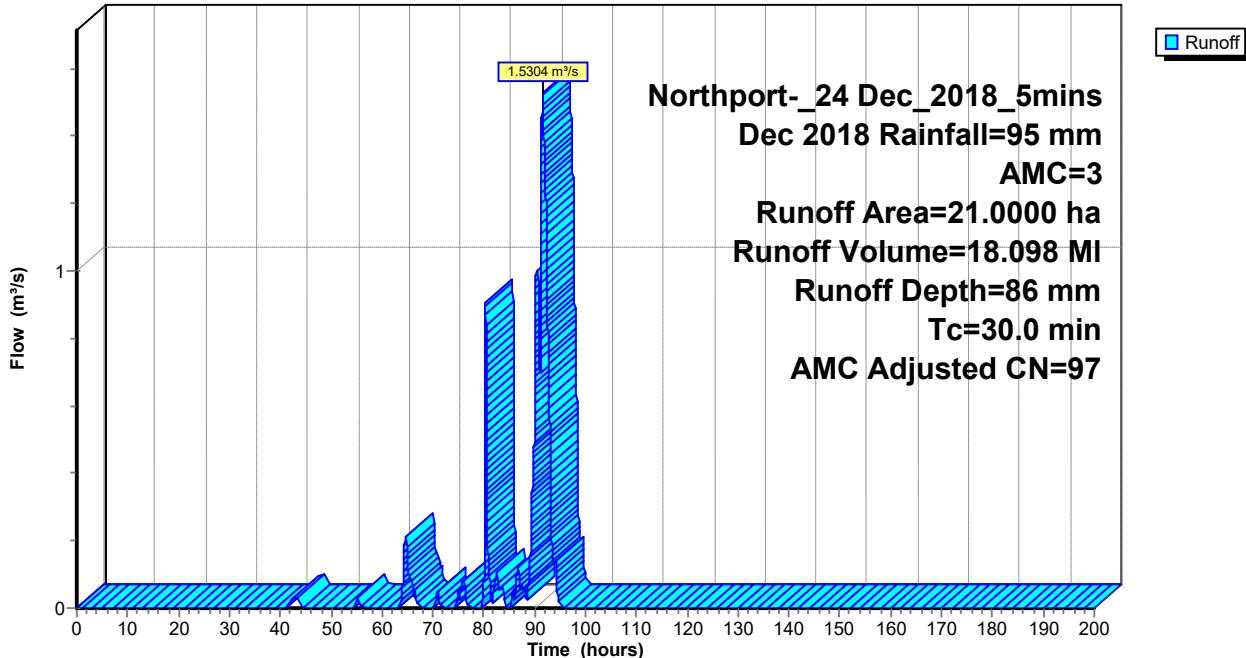
* 21.0000	92		
21.0000	92	97	Weighted Average, AMC Adjusted
21.0000			100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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30.0 Direct Entry,

Subcatchment 72S: Western Catchment

Hydrograph



Summary for Subcatchment 77S: Eastern Catchment (Wood chip pile)

Runoff = 0.0709 m³/s @ 91.63 hrs, Volume= 0.589 MI, Depth= 29 mm
 Routed to Pond 71P : 850dia and canal

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (ha)	CN	Adj	Description
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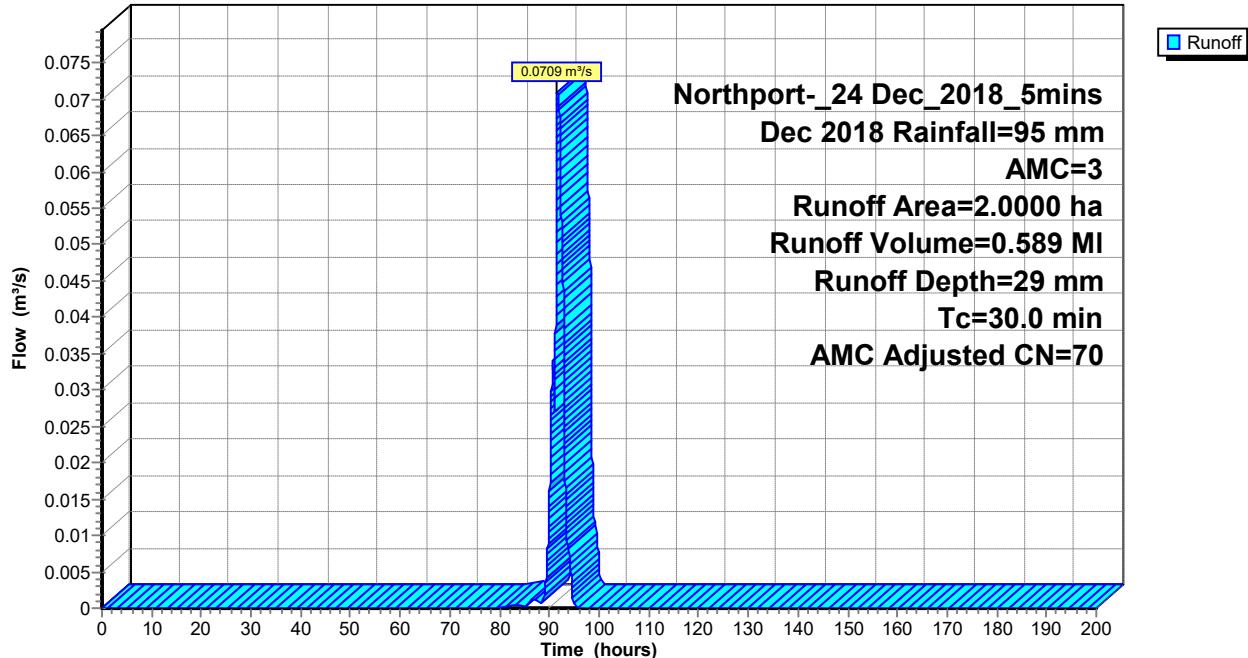
* 2.0000	50		
2.0000	50	70	Weighted Average, AMC Adjusted
2.0000			100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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30.0 Direct Entry,

Subcatchment 77S: Eastern Catchment (Wood chip pile)

Hydrograph



Summary for Subcatchment 79S: Southern Catchment (to sediment ponds)

Runoff = 0.3207 m³/s @ 92.13 hrs, Volume= 3.869 MI, Depth= 70 mm
 Routed to Pond 8P : Canal (Southern) - Dec 2018

Runoff by SCS TR-20 method, UH=SCS, Weighted-CN, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Northport-_24 Dec_2018_5mins Dec 2018 Rainfall=95 mm, AMC=3

Area (ha)	CN	Adj	Description
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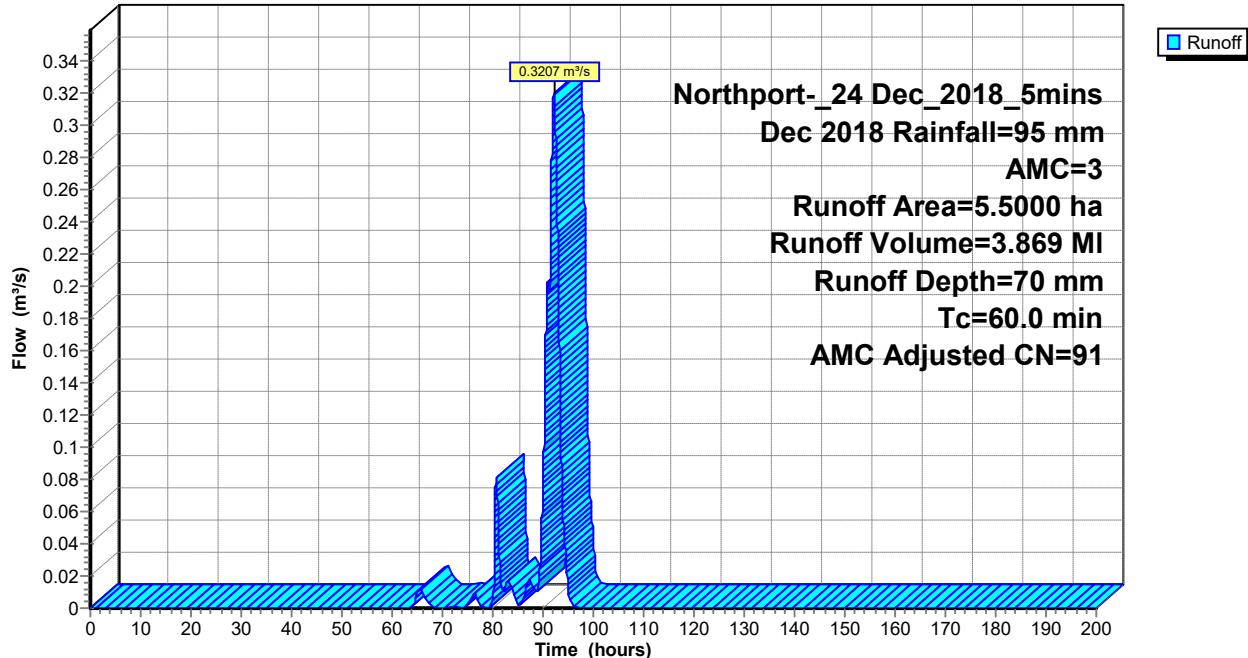
* 5.5000	80		
5.5000	80	91	Weighted Average, AMC Adjusted
5.5000			100.00% Pervious Area

Tc (min)	Length (meters)	Slope (m/m)	Velocity (m/sec)	Capacity (m ³ /s)	Description
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60.0					Direct Entry,
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Subcatchment 79S: Southern Catchment (to sediment ponds)

Hydrograph



Summary for Pond 3P: Pond 2

Inflow Area = 54.7000 ha, 10.42% Impervious, Inflow Depth = 72 mm for Dec 2018 event
 Inflow = 3.0218 m³/s @ 101.01 hrs, Volume= 39.261 MI
 Outflow = 0.2021 m³/s @ 103.58 hrs, Volume= 38.499 MI, Atten= 93%, Lag= 153.9 min
 Discarded = 0.0660 m³/s @ 103.58 hrs, Volume= 14.506 MI
 Primary = 0.1361 m³/s @ 103.58 hrs, Volume= 23.993 MI
 Routed to Pond 65P : 525 Pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 2.975 m Surf.Area= 18,242.3 m² Storage= 2,682.7 m³
 Peak Elev= 3.860 m @ 103.58 hrs Surf.Area= 31,856.0 m² Storage= 28,014.5 m³ (25,331.8 m³ above start)

Plug-Flow detention time= 1,580.7 min calculated for 35.814 MI (91% of inflow)
 Center-of-Mass det. time= 1,225.1 min (7,528.3 - 6,303.2)

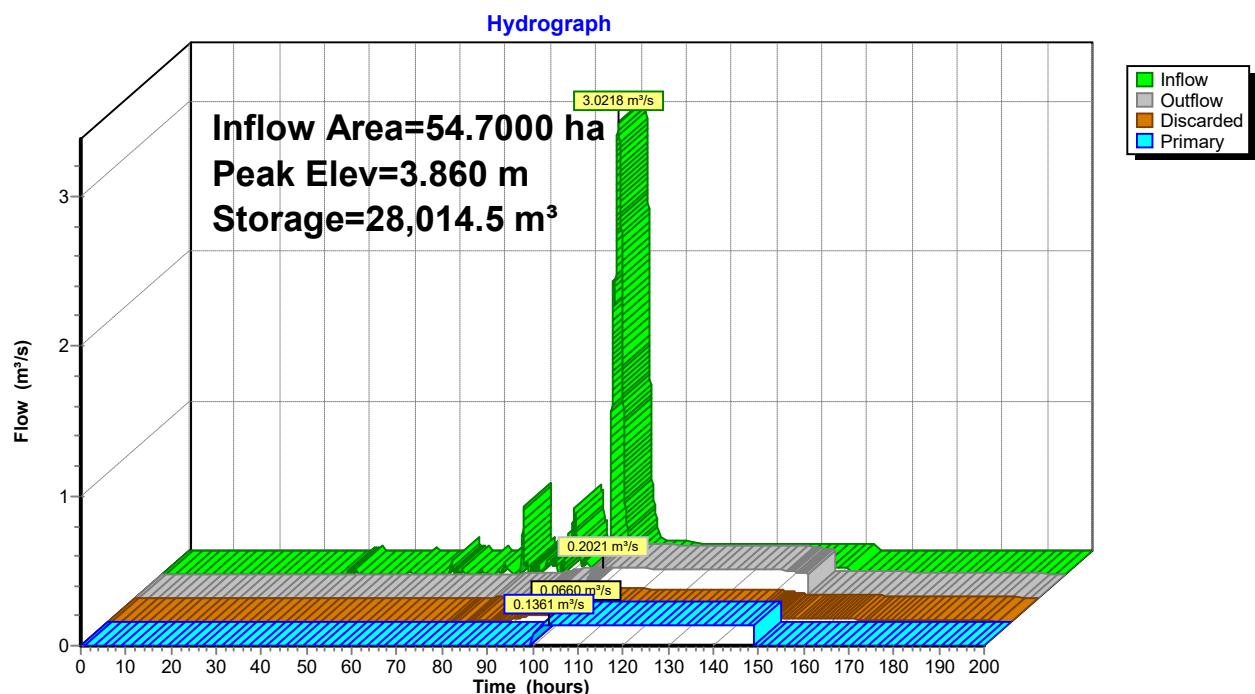
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	81,468.4 m ³	Custom Stage Data (Conic) Listed below
Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)
2.600	474.5	0.0	0.0
2.800	5,907.2	537.1	537.1
3.000	20,004.5	2,452.2	2,989.2
3.200	29,205.2	4,892.0	7,881.3
3.400	29,965.9	5,916.9	13,798.2
3.600	30,727.5	6,069.2	19,867.4
3.800	31,631.9	6,235.7	26,103.1
4.000	32,382.3	6,401.3	32,504.4
4.200	33,112.6	6,549.4	39,053.7
4.400	33,844.5	6,695.6	45,749.3
4.600	34,590.5	6,843.4	52,592.7
4.800	35,339.0	6,992.8	59,585.5
5.000	36,095.3	7,143.3	66,728.8
5.200	36,849.2	7,294.3	74,023.1
5.400	37,605.2	7,445.3	81,468.4
			37,726.9

Device	Routing	Invert	Outlet Devices (Turned on 2 times)
#1	Primary	3.240 m	Pump - Total 290m³/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 4,833.0 4,833.3 Head (meters)= 10.000 0.000 -Loss (meters)= 0.775 0.775 =Lift (meters)= 9.225 -0.775
#2	Primary	3.440 m	Pump - Total 490m³/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 3,333.0 3,333.7 Head (meters)= 10.000 0.000 -Loss (meters)= 0.389 0.389 =Lift (meters)= 9.611 -0.389
#3	Discarded	3.000 m	20.00 mm/hr Exfiltration over Wetted area above 3.000 m Excluded Wetted area = 20,004.8 m ²

Discarded OutFlow Max=0.0660 m³/s @ 103.58 hrs HW=3.860 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0660 m³/s)

Primary OutFlow Max=0.1361 m³/s @ 103.58 hrs HW=3.860 m TW=4.494 m (Dynamic Tailwater)
 ↗ 1=Pump - Total 290m³/hr (Pump Controls 0.0806 m³/s)
 ↗ 2=Pump - Total 490m³/hr (Pump Controls 0.0556 m³/s)

Pond 3P: Pond 2



Summary for Pond 8P: Canal (Southern) - Dec 2018

Inflow Area = 49.0000 ha, 0.00% Impervious, Inflow Depth = 82 mm for Dec 2018 event
 Inflow = 3.2728 m³/s @ 91.57 hrs, Volume= 39.968 MI
 Outflow = 3.1062 m³/s @ 91.85 hrs, Volume= 39.645 MI, Atten= 5%, Lag= 16.5 min
 Discarded = 0.0244 m³/s @ 91.85 hrs, Volume= 3.145 MI
 Primary = 3.0819 m³/s @ 91.85 hrs, Volume= 36.500 MI
 Routed to Link 82L : (new Link)
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.000 MI

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.663 m Surf.Area= 0.5372 ha Storage= 0.325 MI
 Peak Elev= 4.259 m @ 91.85 hrs Surf.Area= 0.9458 ha Storage= 4.743 MI (4.418 MI above start)

Plug-Flow detention time= 210.8 min calculated for 39.321 MI (98% of inflow)
 Center-of-Mass det. time= 187.4 min (5,458.7 - 5,271.4)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15.130 MI	2.60 mW x 1,900.00 mL x 1.50 mH 1900m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 0.5724 ha

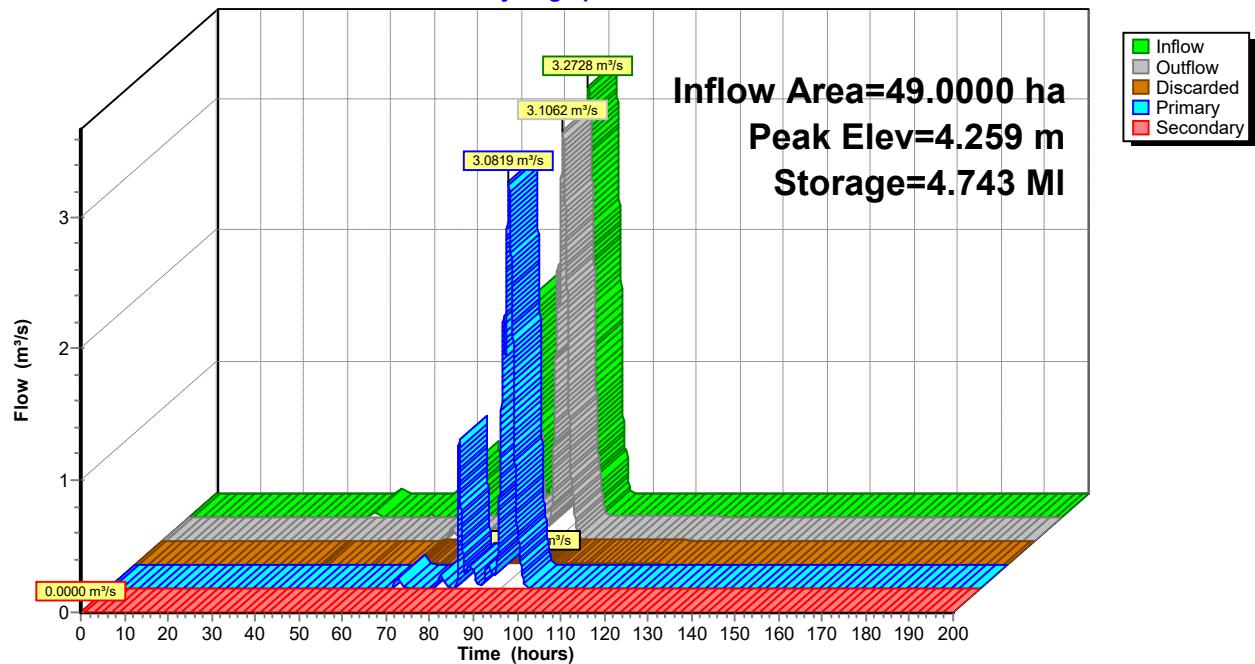
Discarded OutFlow Max=0.0244 m³/s @ 91.85 hrs HW=4.259 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0244 m³/s)

Primary OutFlow Max=3.0818 m³/s @ 91.85 hrs HW=4.259 m TW=0.000 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 3.0818 m³/s @ 1.00 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.663 m (Free Discharge)
 ↗ 2=Overflow (Controls 0.0000 m³/s)

Pond 8P: Canal (Southern) - Dec 2018

Hydrograph



Summary for Pond 59P: 825 pipe

Inflow Area = 54.7000 ha, 10.42% Impervious, Inflow Depth = 44 mm for Dec 2018 event
 Inflow = 0.1365 m³/s @ 100.74 hrs, Volume= 23.990 MI
 Outflow = 0.1364 m³/s @ 100.75 hrs, Volume= 23.987 MI, Atten= 0%, Lag= 0.6 min
 Primary = 0.1364 m³/s @ 100.75 hrs, Volume= 23.987 MI

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.743 m @ 100.75 hrs Surf.Area= 3.1 m² Storage= 3.6 m³

Plug-Flow detention time= 0.5 min calculated for 23.987 MI (100% of inflow)
 Center-of-Mass det. time= 0.3 min (7,482.0 - 7,481.7)

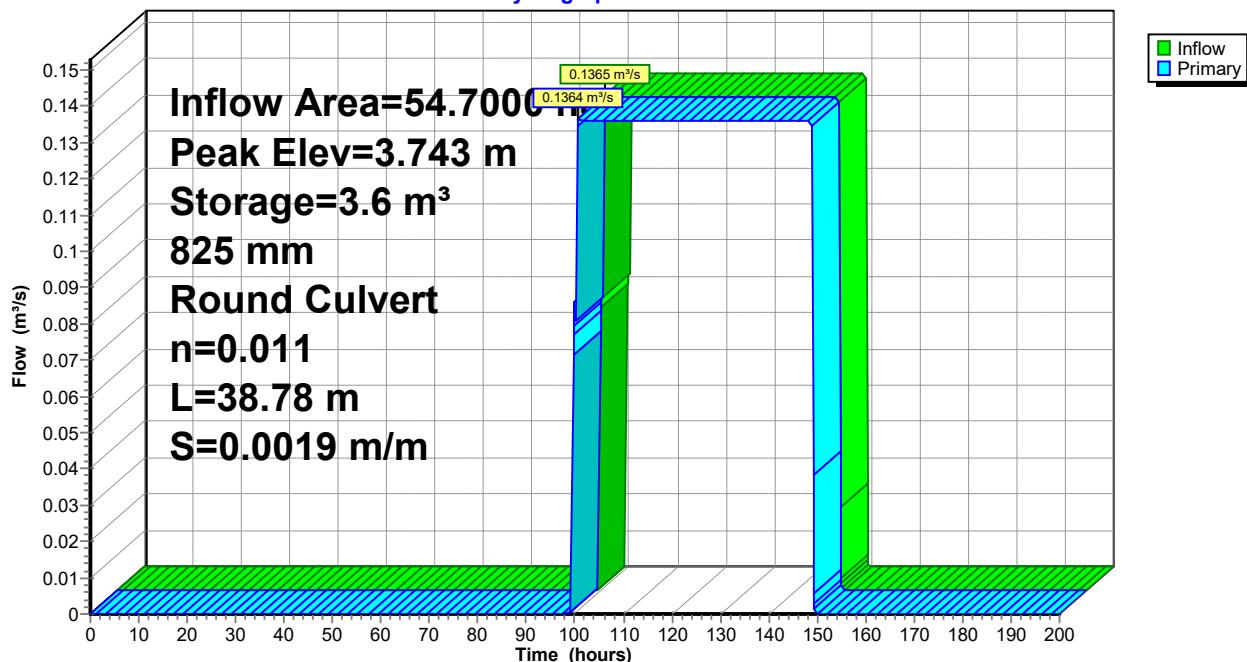
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=0.1364 m³/s @ 100.75 hrs HW=3.743 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.1364 m³/s @ 1.04 m/s)

Pond 59P: 825 pipe

Hydrograph



Summary for Pond 64P: Pond 1

Inflow Area = 51.2000 ha, 4.30% Impervious, Inflow Depth = 75 mm for Dec 2018 event
 Inflow = 3.0819 m³/s @ 100.85 hrs, Volume= 38.524 MI
 Outflow = 3.0371 m³/s @ 101.01 hrs, Volume= 38.000 MI, Atten= 1%, Lag= 9.8 min
 Discarded = 0.0157 m³/s @ 103.58 hrs, Volume= 1.959 MI
 Primary = 2.9718 m³/s @ 101.01 hrs, Volume= 23.428 MI
 Routed to Pond 3P : Pond 2
 Secondary = 0.0500 m³/s @ 75.60 hrs, Volume= 12.613 MI
 Routed to Pond 3P : Pond 2
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.000 MI
 Routed to Pond 59P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.270 m Surf.Area= 17,255.3 m² Storage= 3,381.1 m³
 Peak Elev= 3.860 m @ 103.58 hrs Surf.Area= 20,294.7 m² Storage= 14,522.1 m³ (11,141.0 m³ above start)

Plug-Flow detention time= 677.8 min calculated for 34.617 MI (90% of inflow)
 Center-of-Mass det. time= 547.8 min (6,432.5 - 5,884.7)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0157 m³/s @ 103.58 hrs HW=3.860 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0157 m³/s)

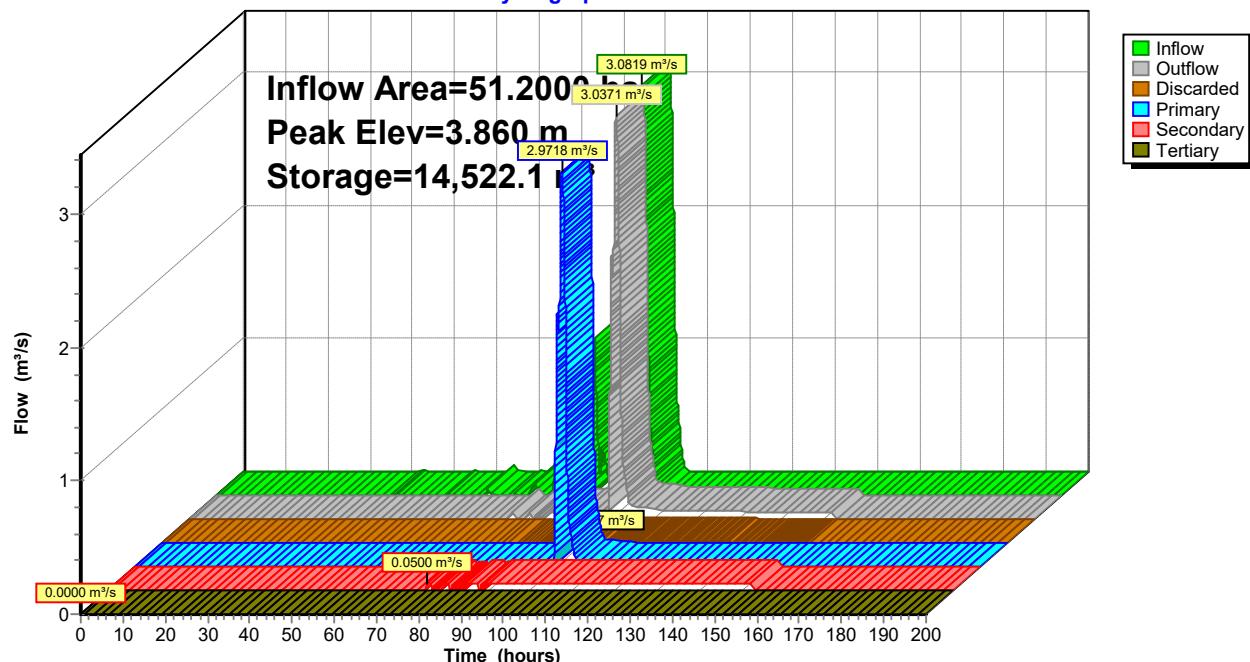
Primary OutFlow Max=2.9717 m³/s @ 101.01 hrs HW=3.843 m TW=3.536 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 2.9717 m³/s @ 0.42 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 75.60 hrs HW=3.300 m TW=3.004 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.270 m TW=2.600 m (Dynamic Tailwater)
 ↗ 4=Culvert (Controls 0.0000 m³/s)
 ↗ 5=Scruffy dome (Controls 0.0000 m³/s)

Pond 64P: Pond 1

Hydrograph



Summary for Pond 65P: 525 Pipe

Inflow Area = 54.7000 ha, 10.42% Impervious, Inflow Depth = 44 mm for Dec 2018 event
 Inflow = 0.1361 m³/s @ 103.58 hrs, Volume= 23.993 MI
 Outflow = 0.1365 m³/s @ 100.74 hrs, Volume= 23.990 MI, Atten= 0%, Lag= 0.0 min
 Primary = 0.1365 m³/s @ 100.74 hrs, Volume= 23.990 MI
 Routed to Pond 59P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.496 m @ 100.74 hrs Surf.Area= 3.1 m² Storage= 6.0 m³

Plug-Flow detention time= 0.8 min calculated for 23.990 MI (100% of inflow)
 Center-of-Mass det. time= 0.5 min (7,481.7 - 7,481.2)

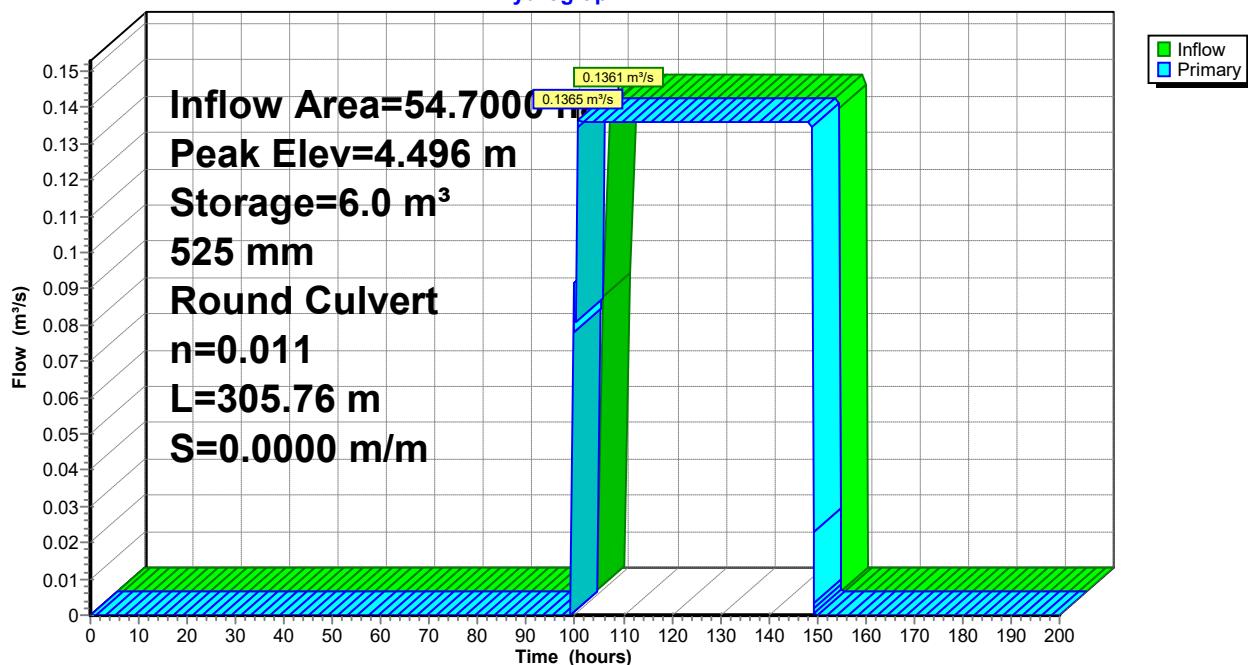
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	12.6 m ³	2.00 mD x 4.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.720 m	525 mm Round Culvert L= 305.76 m Ke= 0.500 Inlet / Outlet Invert= 3.720 m / 3.720 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.216 m ²

Primary OutFlow Max=0.1365 m³/s @ 100.74 hrs HW=4.496 m TW=3.742 m (Dynamic Tailwater)
 ↪=Culvert (Barrel Controls 0.1365 m³/s @ 0.63 m/s)

Pond 65P: 525 Pipe

Hydrograph



Summary for Pond 66P: Canal (Northern)

Inflow Area = 7.0000 ha, 0.00% Impervious, Inflow Depth = 86 mm for Dec 2018 event
 Inflow = 0.5101 m³/s @ 91.56 hrs, Volume= 6.033 MI
 Outflow = 0.4752 m³/s @ 91.75 hrs, Volume= 6.020 MI, Atten= 7%, Lag= 11.3 min
 Primary = 0.4752 m³/s @ 91.75 hrs, Volume= 6.020 MI
 Routed to Pond 8P : Canal (Southern) - Dec 2018
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.000 MI

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.663 m Surf.Area= 212.7 m² Storage= 12.8 m³
 Peak Elev= 4.486 m @ 91.78 hrs Surf.Area= 452.6 m² Storage= 285.3 m³ (272.4 m³ above start)

Plug-Flow detention time= 58.9 min calculated for 6.007 MI (100% of inflow)
 Center-of-Mass det. time= 51.4 min (5,291.6 - 5,240.2)

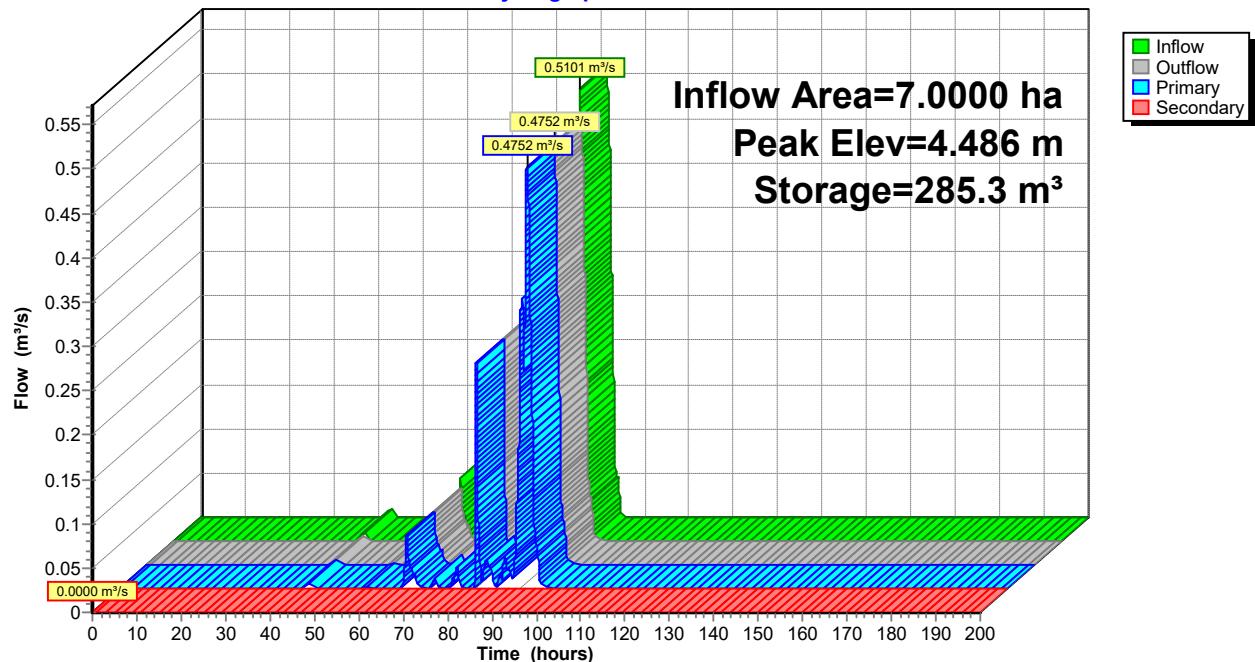
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	621.4 m ³	2.60 mW x 75.00 mL x 1.50 mH 75m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	690 mm Round Culvert L= 35.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.374 m ²
#2	Secondary	5.000 m	75.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83

Primary OutFlow Max=0.4747 m³/s @ 91.75 hrs HW=4.485 m TW=4.256 m (Dynamic Tailwater)
 ↑1=Culvert (Inlet Controls 0.4747 m³/s @ 1.27 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.663 m (Free Discharge)
 ↑2=Overflow (Controls 0.0000 m³/s)

Pond 66P: Canal (Northern)

Hydrograph



Summary for Pond 71P: 850dia and canal

Inflow Area = 6.8500 ha, 0.00% Impervious, Inflow Depth = 70 mm for Dec 2018 event
 Inflow = 0.4241 m³/s @ 91.56 hrs, Volume= 4.769 MI
 Outflow = 0.4221 m³/s @ 91.57 hrs, Volume= 4.769 MI, Atten= 0%, Lag= 0.4 min
 Primary = 0.4221 m³/s @ 91.57 hrs, Volume= 4.769 MI
 Routed to Pond 8P : Canal (Southern) - Dec 2018

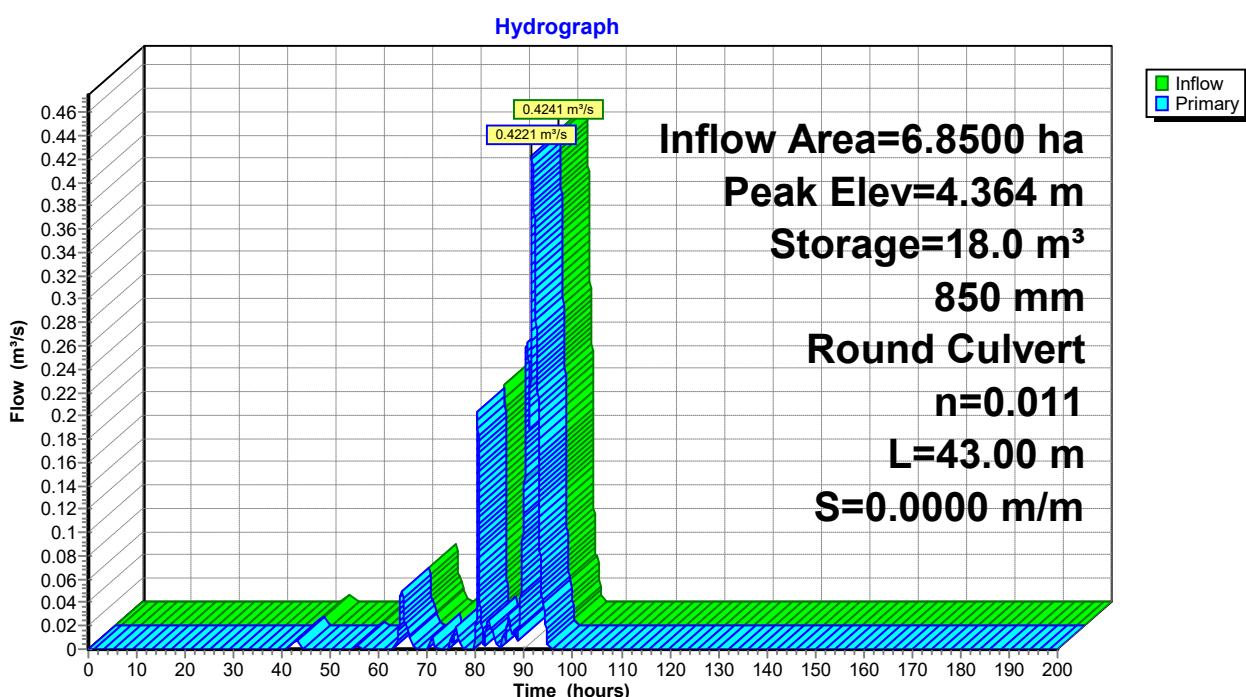
Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.364 m @ 91.72 hrs Surf.Area= 49.5 m² Storage= 18.0 m³

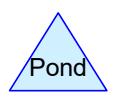
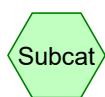
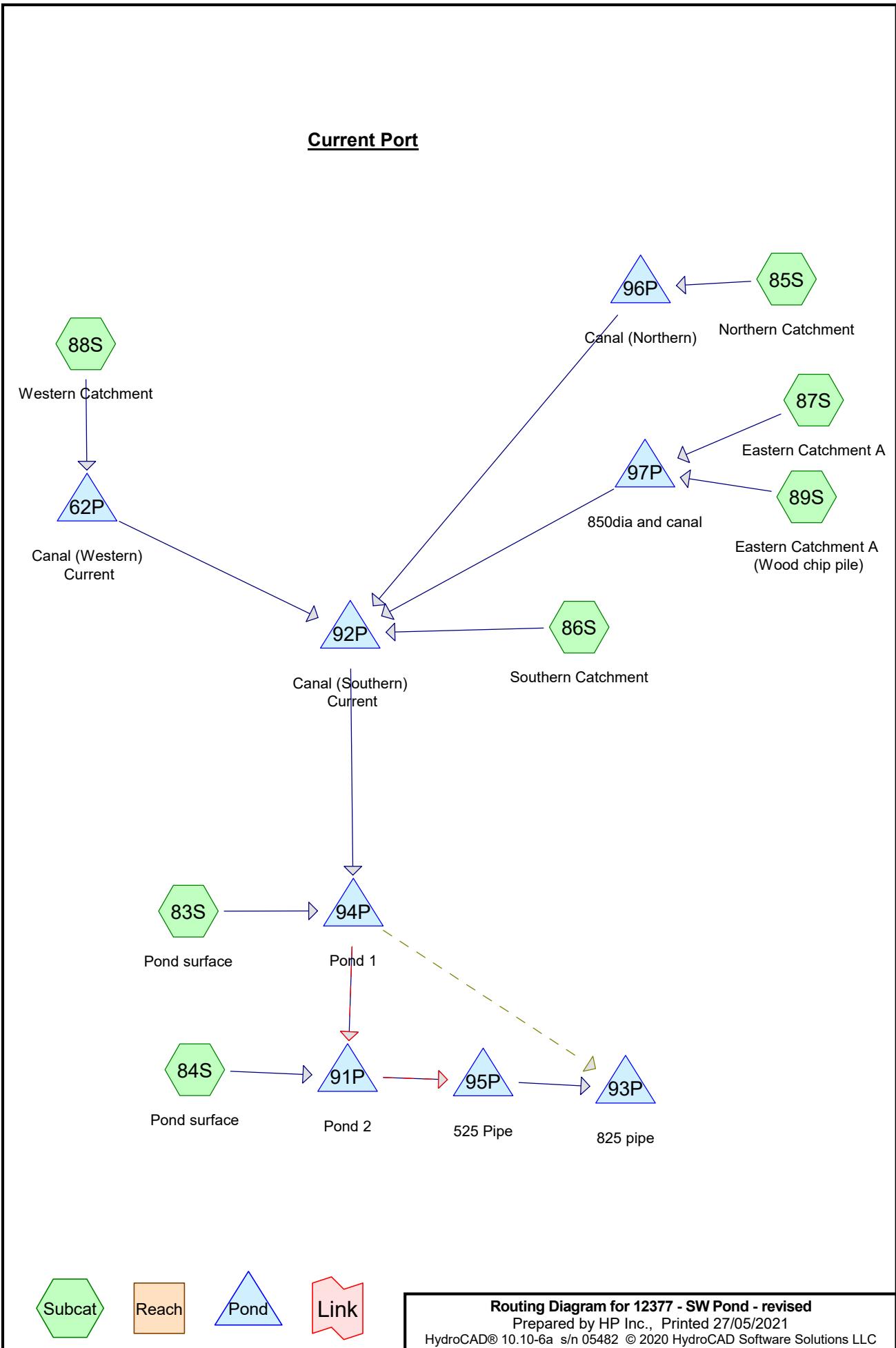
Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 0.7 min (5,272.8 - 5,272.1)

Volume	Invert	Avail.Storage	Storage Description
#1	4.000 m	49.5 m ³	0.45 mW x 110.00 mL x 1.00 mH Prismatoid
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	850 mm Round Culvert L= 43.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.567 m ²

Primary OutFlow Max=0.4186 m³/s @ 91.57 hrs HW=4.356 m TW=4.237 m (Dynamic Tailwater)
 ↪=Culvert (Outlet Controls 0.4186 m³/s @ 1.04 m/s)

Pond 71P: 850dia and canal





Routing Diagram for 12377 - SW Pond - revised

Prepared by HP Inc., Printed 27/05/2021

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Summary for Pond 62P: Canal (Western) Current

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 111 mm for 5yr Type 1A event
 Inflow = 1.5662 m³/s @ 8.20 hrs, Volume= 25,175.5 m³
 Outflow = 1.2150 m³/s @ 8.48 hrs, Volume= 25,009.2 m³, Atten= 22%, Lag= 17.0 min
 Discarded = 0.0116 m³/s @ 8.48 hrs, Volume= 1,003.6 m³
 Primary = 1.2034 m³/s @ 8.48 hrs, Volume= 24,005.7 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.564 m @ 8.48 hrs Surf.Area= 3,541.3 m² Storage= 2,431.0 m³

Plug-Flow detention time= 107.4 min calculated for 25,009.2 m³ (99% of inflow)
 Center-of-Mass det. time= 102.4 min (813.7 - 711.3)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 1,748.1 m ²

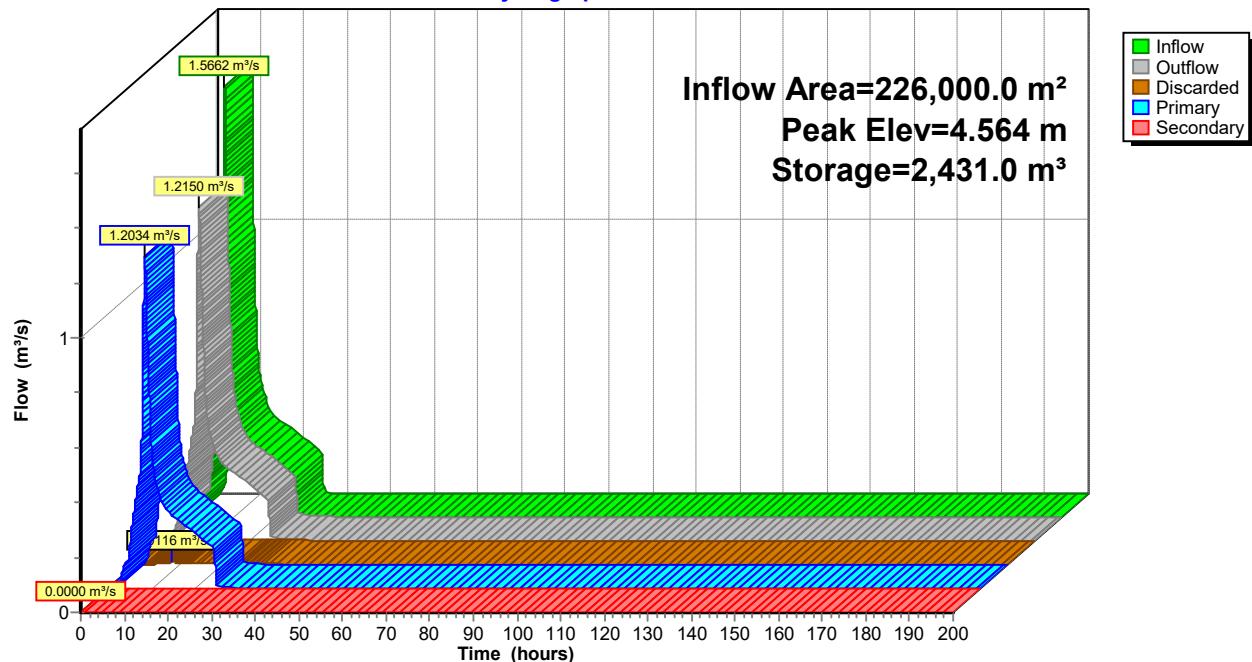
Discarded OutFlow Max=0.0116 m³/s @ 8.48 hrs HW=4.564 m (Free Discharge)
 ↑ 4=Exfiltration (Exfiltration Controls 0.0116 m³/s)

Primary OutFlow Max=1.2033 m³/s @ 8.48 hrs HW=4.564 m TW=4.224 m (Dynamic Tailwater)
 ↑ 1=Culvert (Barrel Controls 1.2033 m³/s @ 1.69 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Spillway (Controls 0.0000 m³/s)
 3=Overflow (Controls 0.0000 m³/s)

Pond 62P: Canal (Western) Current

Hydrograph



Summary for Pond 91P: Pond 2

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 95 mm for 5yr Type 1A event
 Inflow = 2.6050 m³/s @ 8.62 hrs, Volume= 52,197.4 m³
 Outflow = 0.2054 m³/s @ 24.51 hrs, Volume= 54,609.5 m³, Atten= 92%, Lag= 953.9 min
 Discarded = 0.0693 m³/s @ 24.51 hrs, Volume= 19,462.6 m³
 Primary = 0.1361 m³/s @ 24.51 hrs, Volume= 35,146.9 m³
 Routed to Pond 95P : 525 Pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.100 m Surf.Area= 24,604.8 m² Storage= 5,435.2 m³
 Peak Elev= 4.015 m @ 24.51 hrs Surf.Area= 32,438.3 m² Storage= 33,006.7 m³ (27,571.4 m³ above start)

Plug-Flow detention time= 1,919.2 min calculated for 49,170.9 m³ (94% of inflow)
 Center-of-Mass det. time= 1,483.3 min (2,797.0 - 1,313.7)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	81,468.4 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	474.5	0.0	0.0	474.5
2.800	5,907.2	537.1	537.1	5,907.3
3.000	20,004.5	2,452.2	2,989.2	20,004.8
3.200	29,205.2	4,892.0	7,881.3	29,206.2
3.400	29,965.9	5,916.9	13,798.2	29,976.6
3.600	30,727.5	6,069.2	19,867.4	30,748.2
3.800	31,631.9	6,235.7	26,103.1	31,661.2
4.000	32,382.3	6,401.3	32,504.4	32,422.2
4.200	33,112.6	6,549.4	39,053.7	33,163.7
4.400	33,844.5	6,695.6	45,749.3	33,907.0
4.600	34,590.5	6,843.4	52,592.7	34,664.5
4.800	35,339.0	6,992.8	59,585.5	35,424.6
5.000	36,095.3	7,143.3	66,728.8	36,192.7
5.200	36,849.2	7,294.3	74,023.1	36,958.6
5.400	37,605.2	7,445.3	81,468.4	37,726.9

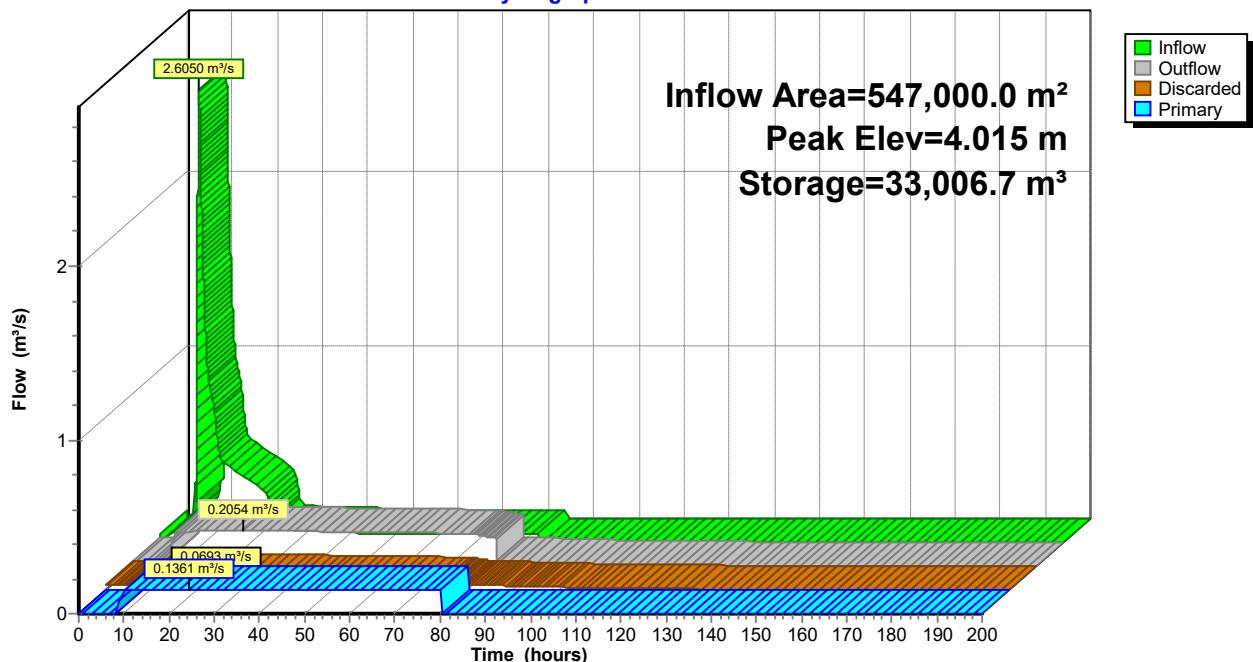
Device	Routing	Invert	Outlet Devices (Turned on 2 times)
#1	Primary	3.240 m	Pump - Total 290m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 4,833.0 4,833.3 Head (meters)= 10.000 0.000 -Loss (meters)= 0.775 0.775 =Lift (meters)= 9.225 -0.775
#2	Primary	3.440 m	Pump - Total 490m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 3,333.0 3,333.7 Head (meters)= 10.000 0.000 -Loss (meters)= 0.389 0.389 =Lift (meters)= 9.611 -0.389
#3	Discarded	3.000 m	20.00 mm/hr Exfiltration over Wetted area above 3.000 m Excluded Wetted area = 20,004.8 m ²

Discarded OutFlow Max=0.0693 m³/s @ 24.51 hrs HW=4.015 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0693 m³/s)

Primary OutFlow Max=0.1361 m³/s @ 24.51 hrs HW=4.015 m TW=4.494 m (Dynamic Tailwater)
 ↗ 1=Pump - Total 290m³/hr (Pump Controls 0.0806 m³/s)
 ↗ 2=Pump - Total 490m³/hr (Pump Controls 0.0556 m³/s)

Pond 91P: Pond 2

Hydrograph



Summary for Pond 92P: Canal (Southern) Current

Inflow Area = 490,000.0 m², 0.00% Impervious, Inflow Depth = 104 mm for 5yr Type 1A event
 Inflow = 2.7081 m³/s @ 8.30 hrs, Volume= 50,833.2 m³
 Outflow = 2.6103 m³/s @ 8.43 hrs, Volume= 50,452.5 m³, Atten= 4%, Lag= 7.9 min
 Discarded = 0.0159 m³/s @ 8.43 hrs, Volume= 2,140.9 m³
 Primary = 2.5944 m³/s @ 8.43 hrs, Volume= 48,311.7 m³
 Routed to Pond 94P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.225 m @ 8.43 hrs Surf.Area= 0.6390 ha Storage= 3.066 MI

Plug-Flow detention time= 113.5 min calculated for 50,450.0 m³ (99% of inflow)
 Center-of-Mass det. time= 104.8 min (844.5 - 739.7)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	10.479 MI	2.60 mW x 1,315.00 mL x 1.50 mH 1315m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 0.3962 ha

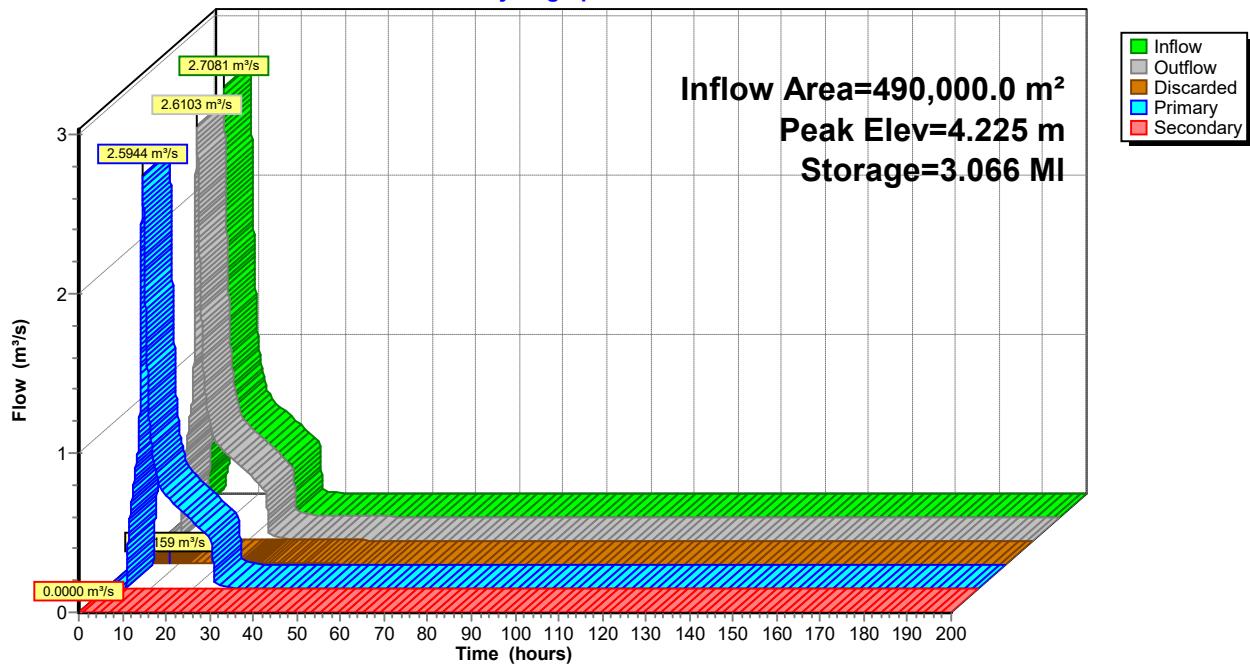
Discarded OutFlow Max=0.0159 m³/s @ 8.43 hrs HW=4.225 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0159 m³/s)

Primary OutFlow Max=2.5943 m³/s @ 8.43 hrs HW=4.225 m TW=3.826 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 2.5943 m³/s @ 0.95 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Pond 92P: Canal (Southern) Current

Hydrograph



Summary for Pond 93P: 825 pipe

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 64 mm for 5yr Type 1A event
 Inflow = 0.1365 m³/s @ 9.23 hrs, Volume= 35,143.4 m³
 Outflow = 0.1364 m³/s @ 9.24 hrs, Volume= 35,140.8 m³, Atten= 0%, Lag= 0.6 min
 Primary = 0.1364 m³/s @ 9.24 hrs, Volume= 35,140.8 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.733 m @ 9.24 hrs Surf.Area= 3.1 m² Storage= 3.6 m³

Plug-Flow detention time= 0.4 min calculated for 35,139.1 m³ (100% of inflow)
 Center-of-Mass det. time= 0.3 min (2,678.2 - 2,677.9)

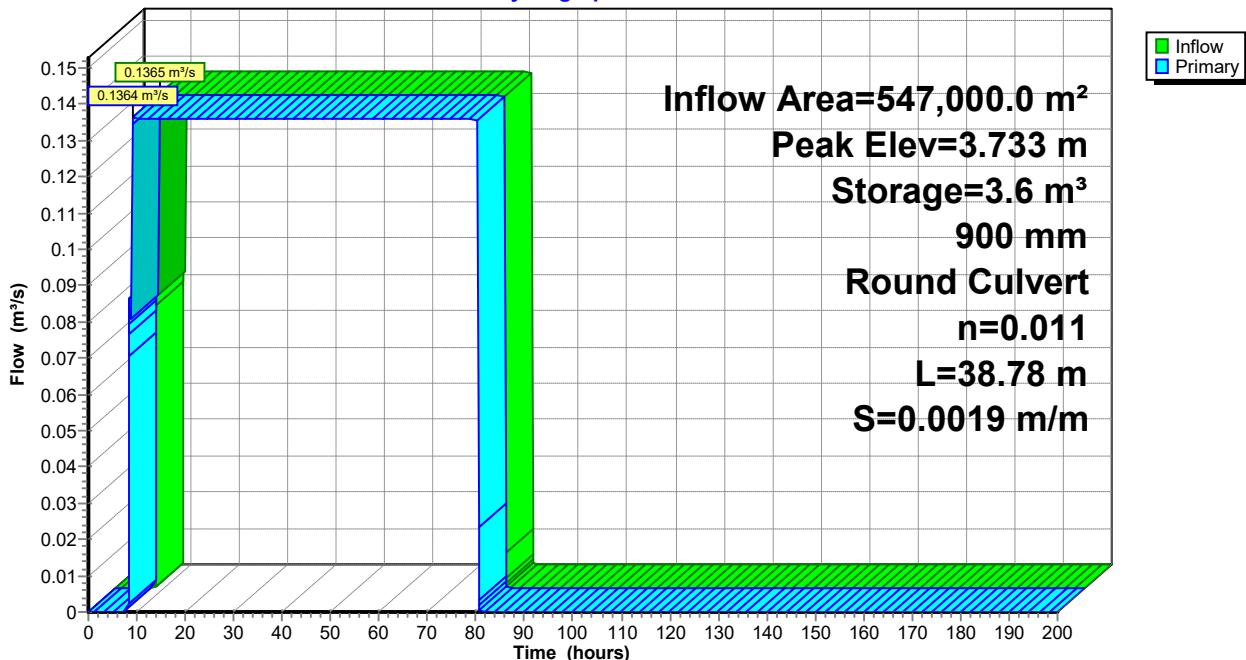
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	900 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.636 m ²

Primary OutFlow Max=0.1364 m³/s @ 9.24 hrs HW=3.733 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.1364 m³/s @ 1.03 m/s)

Pond 93P: 825 pipe

Hydrograph



Summary for Pond 94P: Pond 1

Inflow Area = 512,000.0 m², 4.30% Impervious, Inflow Depth = 100 mm for 5yr Type 1A event
 Inflow = 2.6611 m³/s @ 8.43 hrs, Volume= 51,017.3 m³
 Outflow = 2.5123 m³/s @ 8.62 hrs, Volume= 51,018.5 m³, Atten= 6%, Lag= 11.7 min
 Discarded = 0.0190 m³/s @ 24.52 hrs, Volume= 3,125.5 m³
 Primary = 2.4472 m³/s @ 8.62 hrs, Volume= 33,350.6 m³
 Routed to Pond 91P : Pond 2
 Secondary = 0.0500 m³/s @ 0.00 hrs, Volume= 14,542.5 m³
 Routed to Pond 91P : Pond 2
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Routed to Pond 93P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.300 m Surf.Area= 17,483.7 m² Storage= 3,905.5 m³

Peak Elev= 4.015 m @ 24.52 hrs Surf.Area= 20,884.4 m² Storage= 17,715.2 m³ (13,809.8 m³ above start)

Plug-Flow detention time= 797.4 min calculated for 47,113.1 m³ (92% of inflow)

Center-of-Mass det. time= 663.9 min (1,419.9 - 756.0)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0190 m³/s @ 24.52 hrs HW=4.015 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0190 m³/s)

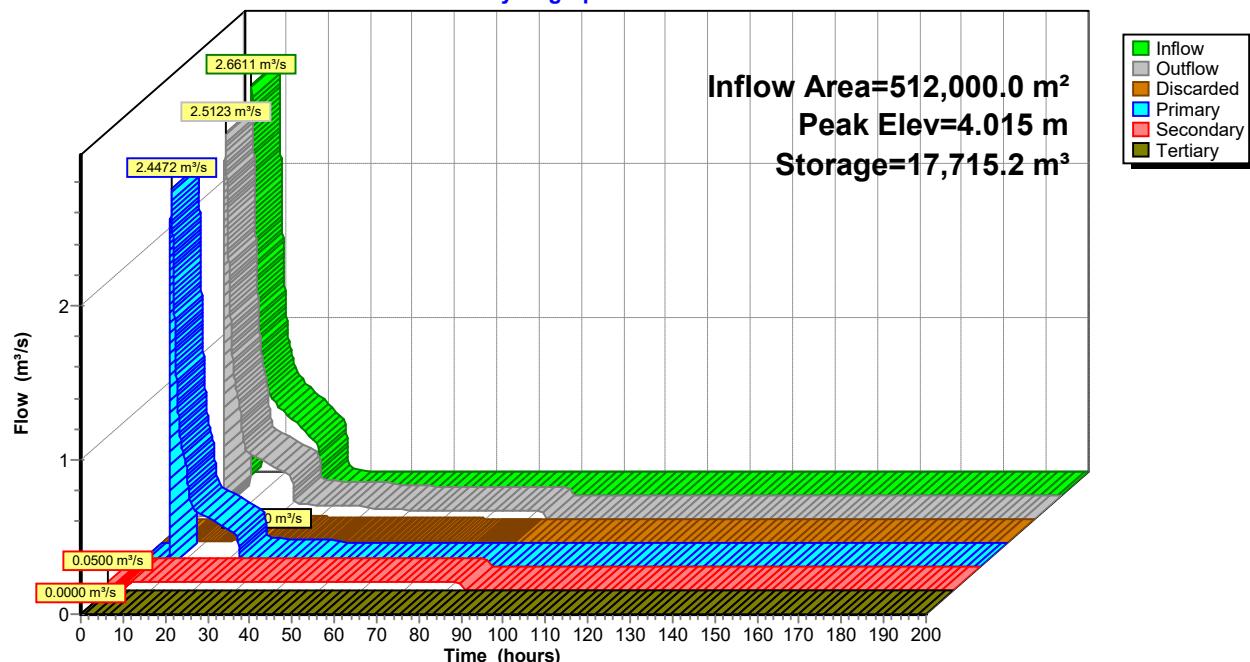
Primary OutFlow Max=2.4471 m³/s @ 8.62 hrs HW=3.832 m TW=3.288 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 2.4471 m³/s @ 0.40 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 0.00 hrs HW=3.300 m TW=3.100 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.300 m TW=2.600 m (Dynamic Tailwater)
 ↗ 4=Culvert (Controls 0.0000 m³/s)
 ↗ 5=Scruffy dome (Controls 0.0000 m³/s)

Pond 94P: Pond 1

Hydrograph



Summary for Pond 96P: Canal (Northern)

Inflow Area = 70,000.0 m², 0.00% Impervious, Inflow Depth = 111 mm for 5yr Type 1A event
 Inflow = 0.4851 m³/s @ 8.20 hrs, Volume= 7,797.7 m³
 Outflow = 0.4510 m³/s @ 8.30 hrs, Volume= 7,776.0 m³, Atten= 7%, Lag= 6.2 min
 Primary = 0.4510 m³/s @ 8.30 hrs, Volume= 7,776.0 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.426 m @ 8.34 hrs Surf.Area= 434.5 m² Storage= 258.6 m³

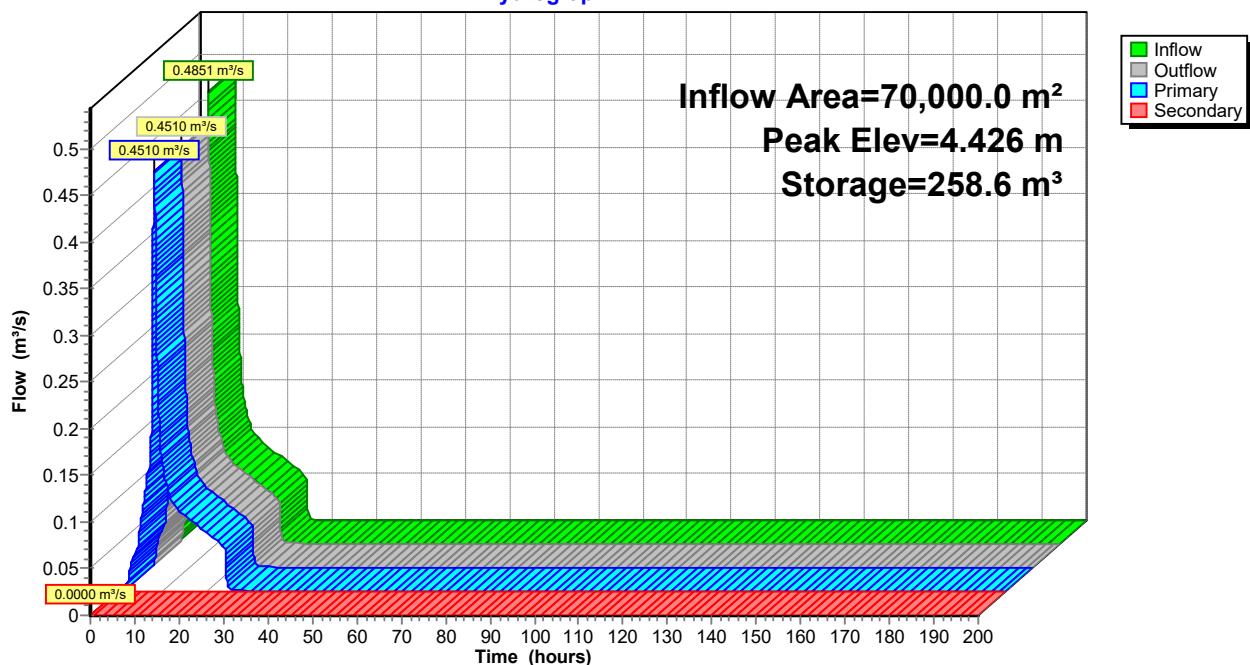
Plug-Flow detention time= 42.8 min calculated for 7,775.6 m³ (100% of inflow)
 Center-of-Mass det. time= 41.1 min (752.4 - 711.3)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	621.4 m ³	2.60 mW x 75.00 mL x 1.50 mH 75m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	690 mm Round Culvert L= 35.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.374 m ²
#2	Secondary	5.000 m	75.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83

Primary OutFlow Max=0.4497 m³/s @ 8.30 hrs HW=4.423 m TW=4.219 m (Dynamic Tailwater)
 ↑1=Culvert (Inlet Controls 0.4497 m³/s @ 1.20 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑2=Overflow (Controls 0.0000 m³/s)

Pond 96P: Canal (Northern)**Hydrograph**

Summary for Pond 62P: Canal (Western) Current

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 134 mm for 10yr Type 1A event
 Inflow = 1.8783 m³/s @ 8.20 hrs, Volume= 30,315.2 m³
 Outflow = 1.4465 m³/s @ 8.49 hrs, Volume= 30,146.8 m³, Atten= 23%, Lag= 17.3 min
 Discarded = 0.0130 m³/s @ 8.49 hrs, Volume= 1,324.4 m³
 Primary = 1.4335 m³/s @ 8.49 hrs, Volume= 28,822.4 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.669 m @ 8.49 hrs Surf.Area= 3,764.6 m² Storage= 2,815.3 m³

Plug-Flow detention time= 118.1 min calculated for 30,145.3 m³ (99% of inflow)
 Center-of-Mass det. time= 114.3 min (818.9 - 704.6)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 1,748.1 m ²

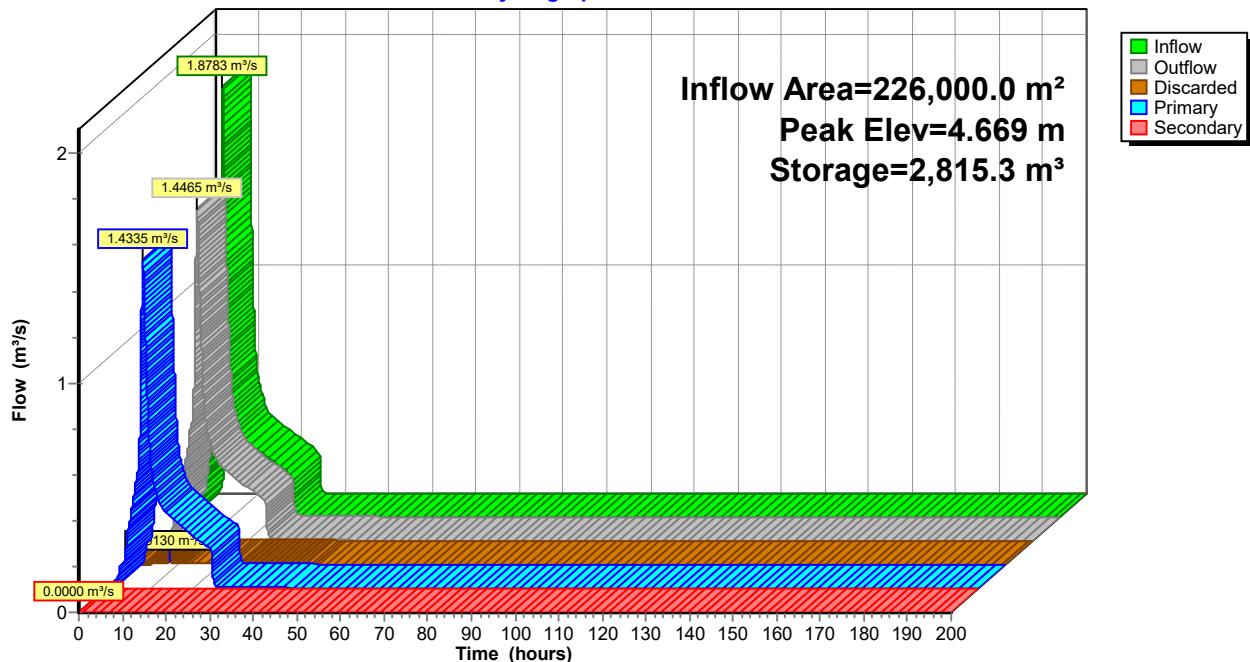
Discarded OutFlow Max=0.0130 m³/s @ 8.49 hrs HW=4.669 m (Free Discharge)
 ↑ 4=Exfiltration (Exfiltration Controls 0.0130 m³/s)

Primary OutFlow Max=1.4334 m³/s @ 8.49 hrs HW=4.669 m TW=4.260 m (Dynamic Tailwater)
 ↑ 1=Culvert (Barrel Controls 1.4334 m³/s @ 1.79 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Spillway (Controls 0.0000 m³/s)
 3=Overflow (Controls 0.0000 m³/s)

Pond 62P: Canal (Western) Current

Hydrograph



Summary for Pond 91P: Pond 2

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 114 mm for 10yr Type 1A event
 Inflow = 3.2335 m³/s @ 8.55 hrs, Volume= 62,184.7 m³
 Outflow = 0.2092 m³/s @ 24.40 hrs, Volume= 64,575.7 m³, Atten= 94%, Lag= 951.0 min
 Discarded = 0.0731 m³/s @ 24.40 hrs, Volume= 22,969.5 m³
 Primary = 0.1361 m³/s @ 24.40 hrs, Volume= 41,606.2 m³

Routed to Pond 95P : 525 Pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.100 m Surf.Area= 24,604.8 m² Storage= 5,435.2 m³

Peak Elev= 4.197 m @ 24.40 hrs Surf.Area= 33,103.2 m² Storage= 38,969.2 m³ (33,534.0 m³ above start)

Plug-Flow detention time= 2,151.2 min calculated for 59,136.6 m³ (95% of inflow)

Center-of-Mass det. time= 1,720.3 min (3,137.9 - 1,417.6)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	81,468.4 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	474.5	0.0	0.0	474.5
2.800	5,907.2	537.1	537.1	5,907.3
3.000	20,004.5	2,452.2	2,989.2	20,004.8
3.200	29,205.2	4,892.0	7,881.3	29,206.2
3.400	29,965.9	5,916.9	13,798.2	29,976.6
3.600	30,727.5	6,069.2	19,867.4	30,748.2
3.800	31,631.9	6,235.7	26,103.1	31,661.2
4.000	32,382.3	6,401.3	32,504.4	32,422.2
4.200	33,112.6	6,549.4	39,053.7	33,163.7
4.400	33,844.5	6,695.6	45,749.3	33,907.0
4.600	34,590.5	6,843.4	52,592.7	34,664.5
4.800	35,339.0	6,992.8	59,585.5	35,424.6
5.000	36,095.3	7,143.3	66,728.8	36,192.7
5.200	36,849.2	7,294.3	74,023.1	36,958.6
5.400	37,605.2	7,445.3	81,468.4	37,726.9

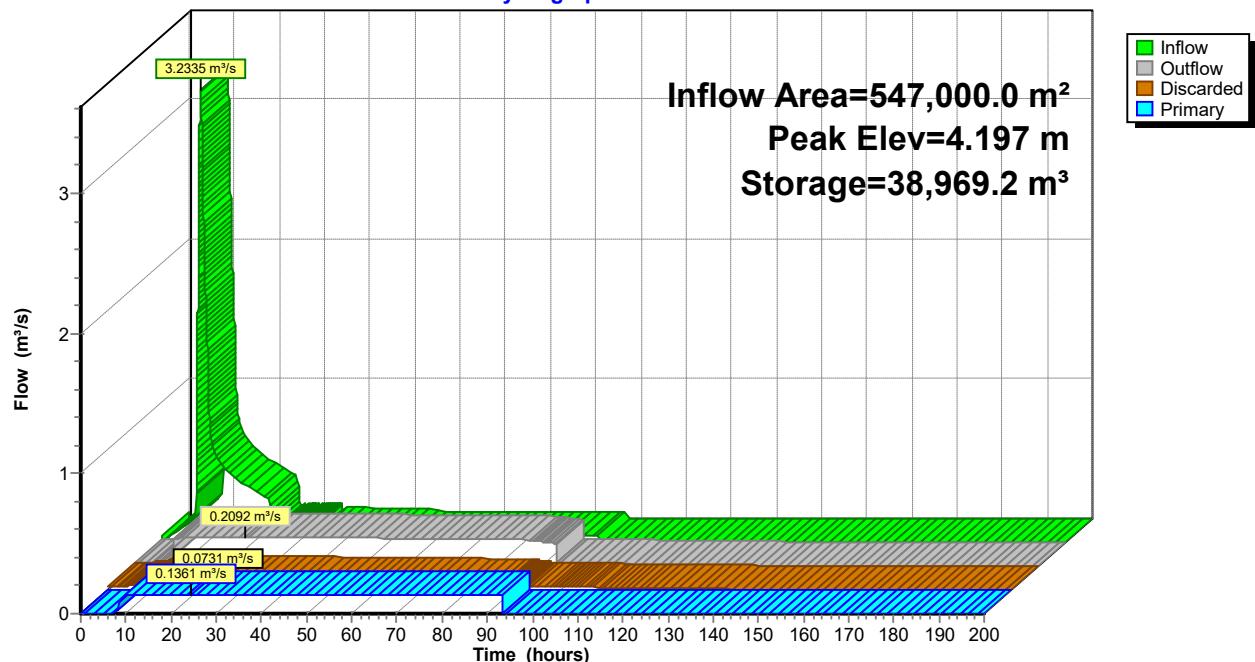
Device	Routing	Invert	Outlet Devices (Turned on 2 times)
#1	Primary	3.240 m	Pump - Total 290m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 4,833.0 4,833.3 Head (meters)= 10.000 0.000 -Loss (meters)= 0.775 0.775 =Lift (meters)= 9.225 -0.775
#2	Primary	3.440 m	Pump - Total 490m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 3,333.0 3,333.7 Head (meters)= 10.000 0.000 -Loss (meters)= 0.389 0.389 =Lift (meters)= 9.611 -0.389
#3	Discarded	3.000 m	20.00 mm/hr Exfiltration over Wetted area above 3.000 m Excluded Wetted area = 20,004.8 m ²

Discarded OutFlow Max=0.0731 m³/s @ 24.40 hrs HW=4.197 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0731 m³/s)

Primary OutFlow Max=0.1361 m³/s @ 24.40 hrs HW=4.197 m TW=4.494 m (Dynamic Tailwater)
 ↗ 1=Pump - Total 290m³/hr (Pump Controls 0.0806 m³/s)
 ↗ 2=Pump - Total 490m³/hr (Pump Controls 0.0556 m³/s)

Pond 91P: Pond 2

Hydrograph



Summary for Pond 92P: Canal (Southern) Current

Inflow Area = 490,000.0 m², 0.00% Impervious, Inflow Depth = 125 mm for 10yr Type 1A event
 Inflow = 3.2446 m³/s @ 8.30 hrs, Volume= 61,382.8 m³
 Outflow = 3.1347 m³/s @ 8.43 hrs, Volume= 60,996.2 m³, Atten= 3%, Lag= 7.8 min
 Discarded = 0.0170 m³/s @ 8.43 hrs, Volume= 2,839.4 m³
 Primary = 3.1177 m³/s @ 8.43 hrs, Volume= 58,156.9 m³
 Routed to Pond 94P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.261 m @ 8.43 hrs Surf.Area= 0.6561 ha Storage= 3.299 MI

Plug-Flow detention time= 128.5 min calculated for 60,993.2 m³ (99% of inflow)
 Center-of-Mass det. time= 117.5 min (851.4 - 733.9)

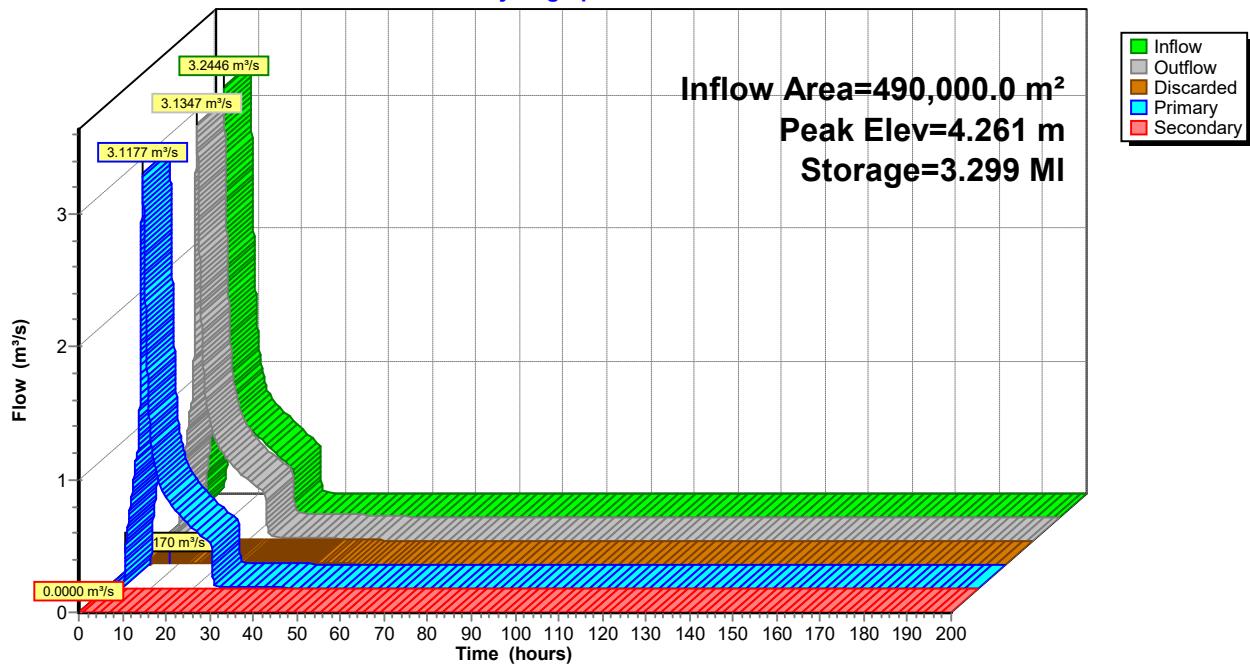
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	10.479 MI	2.60 mW x 1,315.00 mL x 1.50 mH 1315m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 0.3962 ha

Discarded OutFlow Max=0.0170 m³/s @ 8.43 hrs HW=4.261 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0170 m³/s)

Primary OutFlow Max=3.1176 m³/s @ 8.43 hrs HW=4.261 m TW=3.843 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 3.1176 m³/s @ 1.01 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Pond 92P: Canal (Southern) Current**Hydrograph**

Summary for Pond 93P: 825 pipe

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 76 mm for 10yr Type 1A event
 Inflow = 0.1365 m³/s @ 8.69 hrs, Volume= 41,602.7 m³
 Outflow = 0.1364 m³/s @ 8.70 hrs, Volume= 41,600.1 m³, Atten= 0%, Lag= 0.6 min
 Primary = 0.1364 m³/s @ 8.70 hrs, Volume= 41,600.1 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.733 m @ 8.70 hrs Surf.Area= 3.1 m² Storage= 3.6 m³

Plug-Flow detention time= 0.4 min calculated for 41,598.0 m³ (100% of inflow)
 Center-of-Mass det. time= 0.3 min (3,046.6 - 3,046.3)

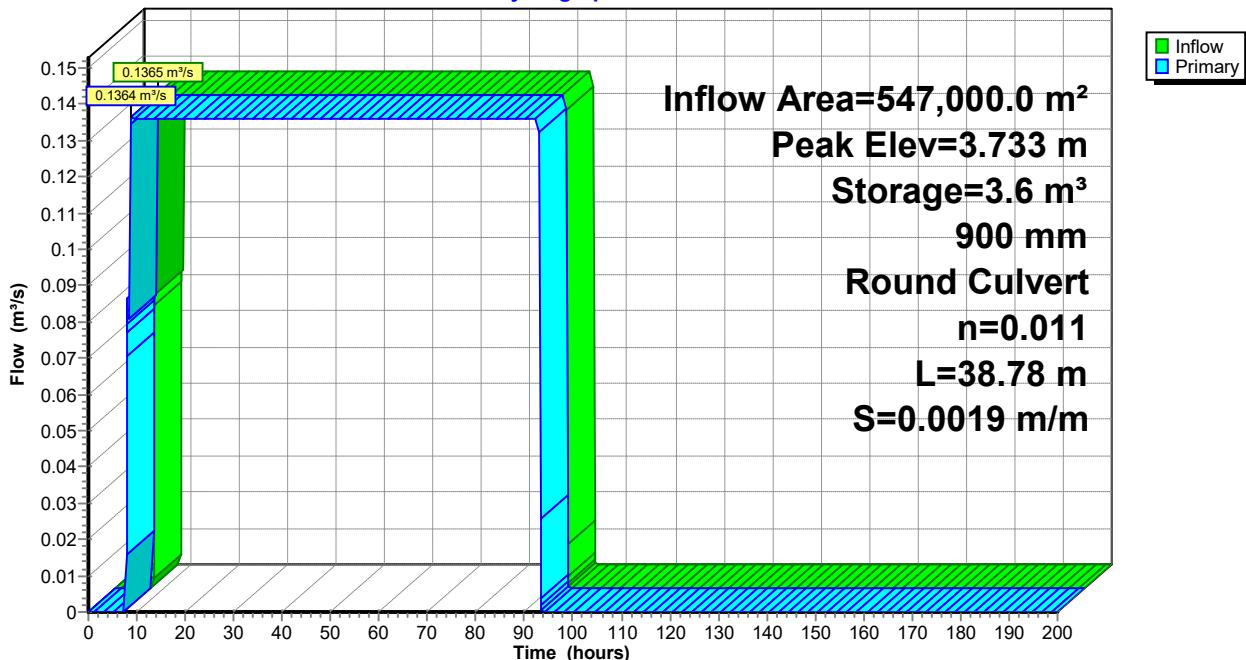
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	900 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.636 m ²

Primary OutFlow Max=0.1364 m³/s @ 8.70 hrs HW=3.733 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.1364 m³/s @ 1.03 m/s)

Pond 93P: 825 pipe

Hydrograph



Summary for Pond 94P: Pond 1

Inflow Area = 512,000.0 m², 4.30% Impervious, Inflow Depth = 120 mm for 10yr Type 1A event
 Inflow = 3.1965 m³/s @ 8.42 hrs, Volume= 61,367.8 m³
 Outflow = 3.1161 m³/s @ 8.56 hrs, Volume= 61,361.4 m³, Atten= 3%, Lag= 7.9 min
 Discarded = 0.0229 m³/s @ 24.41 hrs, Volume= 4,285.0 m³
 Primary = 3.0507 m³/s @ 8.56 hrs, Volume= 40,146.6 m³
 Routed to Pond 91P : Pond 2
 Secondary = 0.0500 m³/s @ 0.00 hrs, Volume= 16,929.8 m³
 Routed to Pond 91P : Pond 2
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Routed to Pond 93P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.300 m Surf.Area= 17,483.7 m² Storage= 3,905.5 m³

Peak Elev= 4.197 m @ 24.41 hrs Surf.Area= 21,571.0 m² Storage= 21,575.8 m³ (17,670.3 m³ above start)

Plug-Flow detention time= 929.8 min calculated for 57,456.0 m³ (94% of inflow)

Center-of-Mass det. time= 798.5 min (1,549.5 - 751.0)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0229 m³/s @ 24.41 hrs HW=4.197 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0229 m³/s)

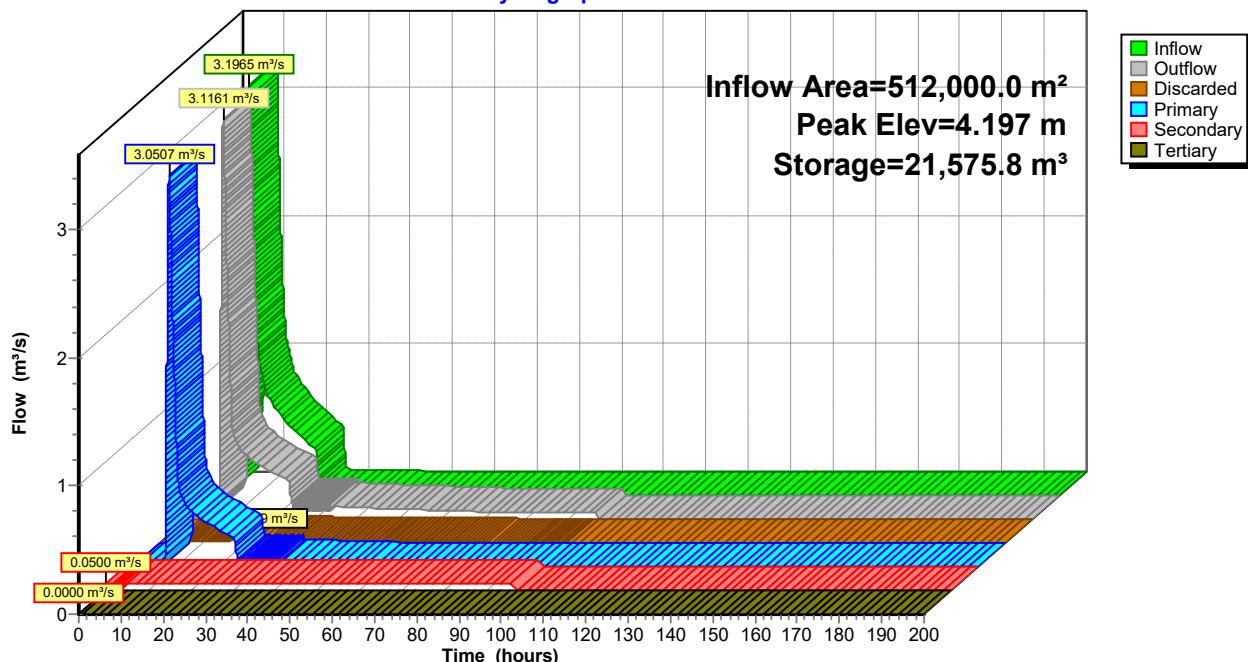
Primary OutFlow Max=3.0505 m³/s @ 8.56 hrs HW=3.845 m TW=3.400 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 3.0505 m³/s @ 0.43 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 0.00 hrs HW=3.300 m TW=3.100 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.300 m TW=2.600 m (Dynamic Tailwater)
 ↗ 4=Culvert (Controls 0.0000 m³/s)
 ↗ 5=Scruffy dome (Controls 0.0000 m³/s)

Pond 94P: Pond 1

Hydrograph



Summary for Pond 96P: Canal (Northern)

Inflow Area = 70,000.0 m², 0.00% Impervious, Inflow Depth = 134 mm for 10yr Type 1A event
 Inflow = 0.5818 m³/s @ 8.20 hrs, Volume= 9,389.7 m³
 Outflow = 0.5326 m³/s @ 8.32 hrs, Volume= 9,367.6 m³, Atten= 8%, Lag= 7.5 min
 Primary = 0.5326 m³/s @ 8.32 hrs, Volume= 9,367.6 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.544 m @ 8.35 hrs Surf.Area= 470.2 m² Storage= 312.1 m³

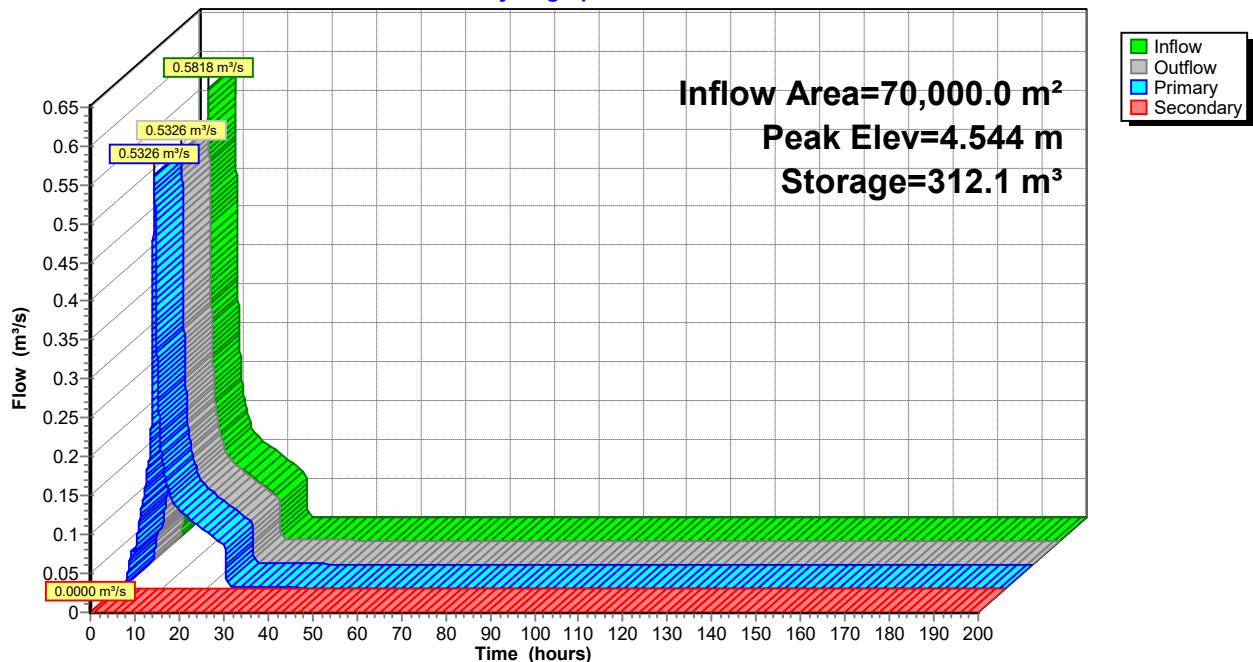
Plug-Flow detention time= 47.5 min calculated for 9,367.1 m³ (100% of inflow)
 Center-of-Mass det. time= 46.1 min (750.8 - 704.6)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	621.4 m ³	2.60 mW x 75.00 mL x 1.50 mH 75m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	690 mm Round Culvert L= 35.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.374 m ²
#2	Secondary	5.000 m	75.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83

Primary OutFlow Max=0.5316 m³/s @ 8.32 hrs HW=4.542 m TW=4.256 m (Dynamic Tailwater)
 ↑1=Culvert (Inlet Controls 0.5316 m³/s @ 1.42 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑2=Overflow (Controls 0.0000 m³/s)

Pond 96P: Canal (Northern)**Hydrograph**

Summary for Pond 62P: Canal (Western) Current

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 158 mm for 20yr Type 1A event
 Inflow = 2.2021 m³/s @ 8.20 hrs, Volume= 35,693.5 m³
 Outflow = 1.8077 m³/s @ 8.44 hrs, Volume= 35,522.1 m³, Atten= 18%, Lag= 14.5 min
 Discarded = 0.0142 m³/s @ 8.44 hrs, Volume= 1,771.6 m³
 Primary = 1.6264 m³/s @ 8.44 hrs, Volume= 33,584.9 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.1672 m³/s @ 8.44 hrs, Volume= 165.5 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.756 m @ 8.44 hrs Surf.Area= 3,949.2 m² Storage= 3,150.0 m³

Plug-Flow detention time= 137.8 min calculated for 35,520.3 m³ (100% of inflow)
 Center-of-Mass det. time= 134.6 min (833.8 - 699.2)

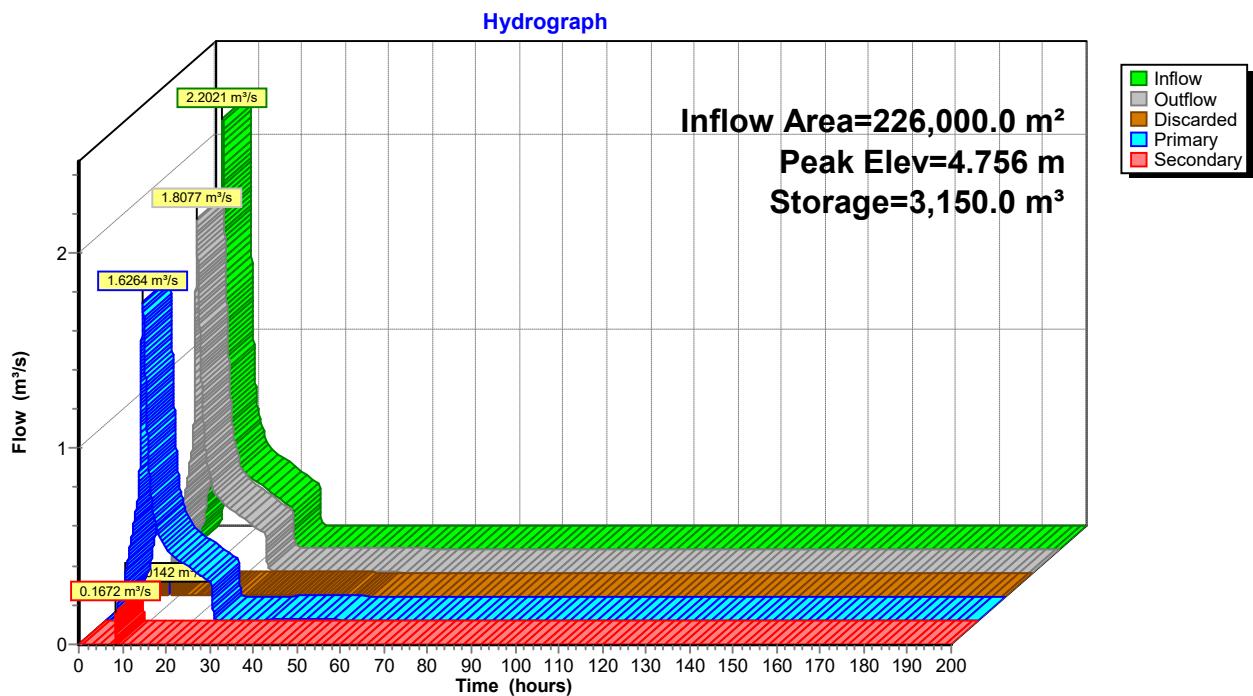
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 1,748.1 m ²

Discarded OutFlow Max=0.0142 m³/s @ 8.44 hrs HW=4.756 m (Free Discharge)
 ↑ 4=Exfiltration (Exfiltration Controls 0.0142 m³/s)

Primary OutFlow Max=1.6264 m³/s @ 8.44 hrs HW=4.756 m TW=4.296 m (Dynamic Tailwater)
 ↑ 1=Culvert (Barrel Controls 1.6264 m³/s @ 1.86 m/s)

Secondary OutFlow Max=0.1671 m³/s @ 8.44 hrs HW=4.756 m (Free Discharge)
 ↑ 2=Spillway (Weir Controls 0.1671 m³/s @ 0.32 m/s)
 3=Overflow (Controls 0.0000 m³/s)

Pond 62P: Canal (Western) Current

Summary for Pond 91P: Pond 2

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 132 mm for 20yr Type 1A event
 Inflow = 3.7930 m³/s @ 8.53 hrs, Volume= 72,039.8 m³
 Outflow = 0.2128 m³/s @ 24.40 hrs, Volume= 74,397.8 m³, Atten= 94%, Lag= 952.5 min
 Discarded = 0.0767 m³/s @ 24.40 hrs, Volume= 26,524.3 m³
 Primary = 0.1361 m³/s @ 24.40 hrs, Volume= 47,873.5 m³
 Routed to Pond 95P : 525 Pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.100 m Surf.Area= 24,604.8 m² Storage= 5,435.2 m³
 Peak Elev= 4.376 m @ 24.40 hrs Surf.Area= 33,755.5 m² Storage= 44,934.7 m³ (39,499.5 m³ above start)

Plug-Flow detention time= 2,370.3 min calculated for 68,958.2 m³ (96% of inflow)
 Center-of-Mass det. time= 1,943.0 min (3,471.7 - 1,528.7)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	81,468.4 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	474.5	0.0	0.0	474.5
2.800	5,907.2	537.1	537.1	5,907.3
3.000	20,004.5	2,452.2	2,989.2	20,004.8
3.200	29,205.2	4,892.0	7,881.3	29,206.2
3.400	29,965.9	5,916.9	13,798.2	29,976.6
3.600	30,727.5	6,069.2	19,867.4	30,748.2
3.800	31,631.9	6,235.7	26,103.1	31,661.2
4.000	32,382.3	6,401.3	32,504.4	32,422.2
4.200	33,112.6	6,549.4	39,053.7	33,163.7
4.400	33,844.5	6,695.6	45,749.3	33,907.0
4.600	34,590.5	6,843.4	52,592.7	34,664.5
4.800	35,339.0	6,992.8	59,585.5	35,424.6
5.000	36,095.3	7,143.3	66,728.8	36,192.7
5.200	36,849.2	7,294.3	74,023.1	36,958.6
5.400	37,605.2	7,445.3	81,468.4	37,726.9

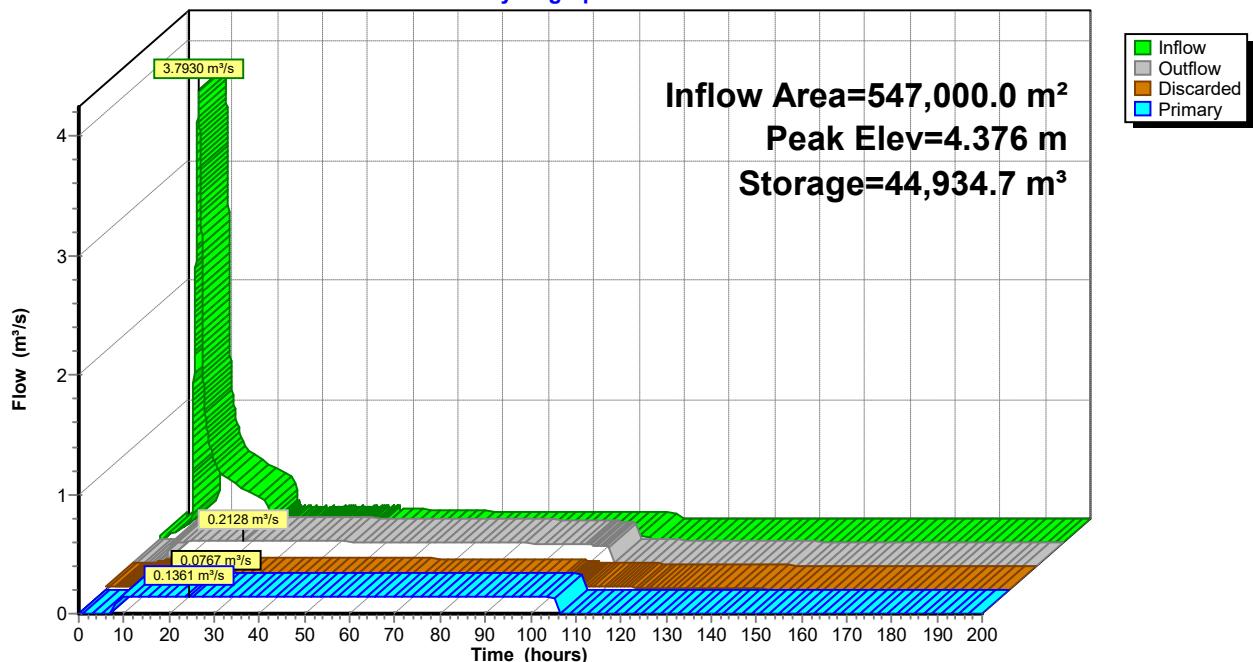
Device	Routing	Invert	Outlet Devices (Turned on 2 times)
#1	Primary	3.240 m	Pump - Total 290m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 4,833.0 4,833.3 Head (meters)= 10.000 0.000 -Loss (meters)= 0.775 0.775 =Lift (meters)= 9.225 -0.775
#2	Primary	3.440 m	Pump - Total 490m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 3,333.0 3,333.7 Head (meters)= 10.000 0.000 -Loss (meters)= 0.389 0.389 =Lift (meters)= 9.611 -0.389
#3	Discarded	3.000 m	20.00 mm/hr Exfiltration over Wetted area above 3.000 m Excluded Wetted area = 20,004.8 m ²

Discarded OutFlow Max=0.0767 m³/s @ 24.40 hrs HW=4.376 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0767 m³/s)

Primary OutFlow Max=0.1361 m³/s @ 24.40 hrs HW=4.376 m TW=4.494 m (Dynamic Tailwater)
 ↗ 1=Pump - Total 290m³/hr (Pump Controls 0.0806 m³/s)
 ↗ 2=Pump - Total 490m³/hr (Pump Controls 0.0556 m³/s)

Pond 91P: Pond 2

Hydrograph



Summary for Pond 92P: Canal (Southern) Current

Inflow Area = 490,000.0 m², 0.00% Impervious, Inflow Depth = 147 mm for 20yr Type 1A event
 Inflow = 3.7949 m³/s @ 8.30 hrs, Volume= 72,182.2 m³
 Outflow = 3.6654 m³/s @ 8.42 hrs, Volume= 71,787.5 m³, Atten= 3%, Lag= 7.1 min
 Discarded = 0.0204 m³/s @ 24.40 hrs, Volume= 3,832.8 m³
 Primary = 3.6474 m³/s @ 8.42 hrs, Volume= 67,954.8 m³
 Routed to Pond 94P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.376 m @ 24.40 hrs Surf.Area= 0.7107 ha Storage= 4.082 MI

Plug-Flow detention time= 154.5 min calculated for 71,787.5 m³ (99% of inflow)
 Center-of-Mass det. time= 140.6 min (873.3 - 732.7)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	10.479 MI	2.60 mW x 1,315.00 mL x 1.50 mH 1315m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 0.3962 ha

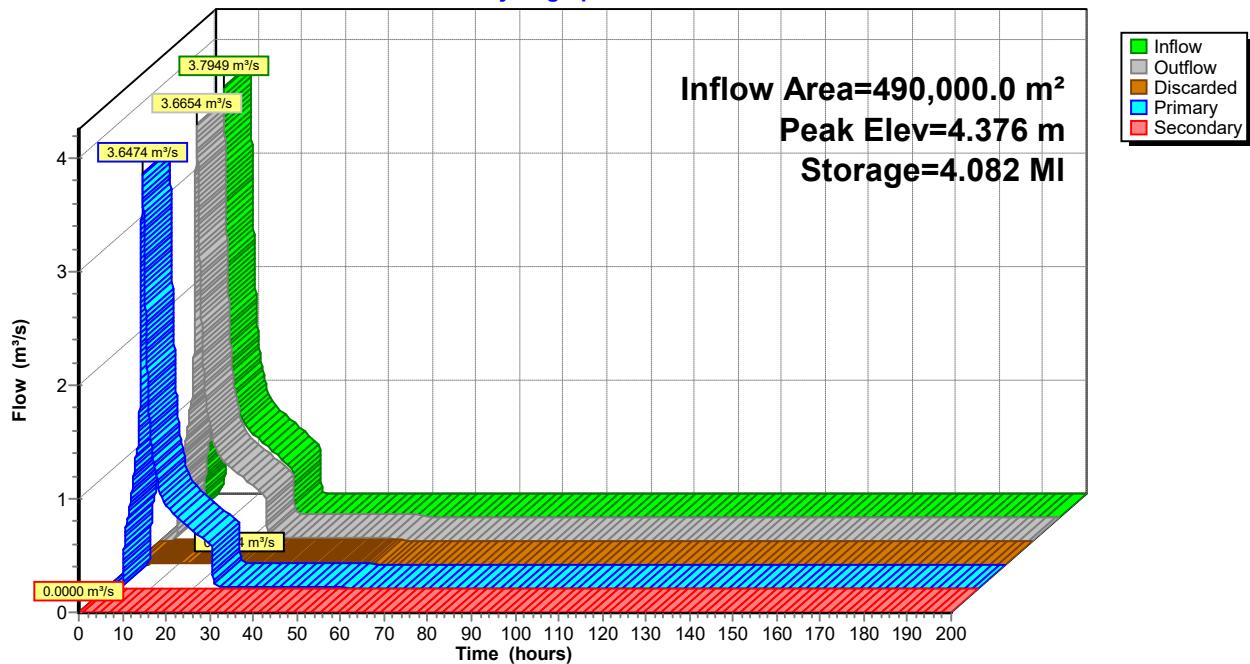
Discarded OutFlow Max=0.0204 m³/s @ 24.40 hrs HW=4.376 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0204 m³/s)

Primary OutFlow Max=3.6473 m³/s @ 8.42 hrs HW=4.296 m TW=3.854 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 3.6473 m³/s @ 1.06 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Pond 92P: Canal (Southern) Current

Hydrograph



Summary for Pond 93P: 825 pipe

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 88 mm for 20yr Type 1A event
 Inflow = 0.1365 m³/s @ 8.35 hrs, Volume= 47,869.9 m³
 Outflow = 0.1364 m³/s @ 8.36 hrs, Volume= 47,867.4 m³, Atten= 0%, Lag= 0.6 min
 Primary = 0.1364 m³/s @ 8.36 hrs, Volume= 47,867.4 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.733 m @ 8.36 hrs Surf.Area= 3.1 m² Storage= 3.6 m³

Plug-Flow detention time= 0.4 min calculated for 47,865.0 m³ (100% of inflow)
 Center-of-Mass det. time= 0.3 min (3,406.7 - 3,406.4)

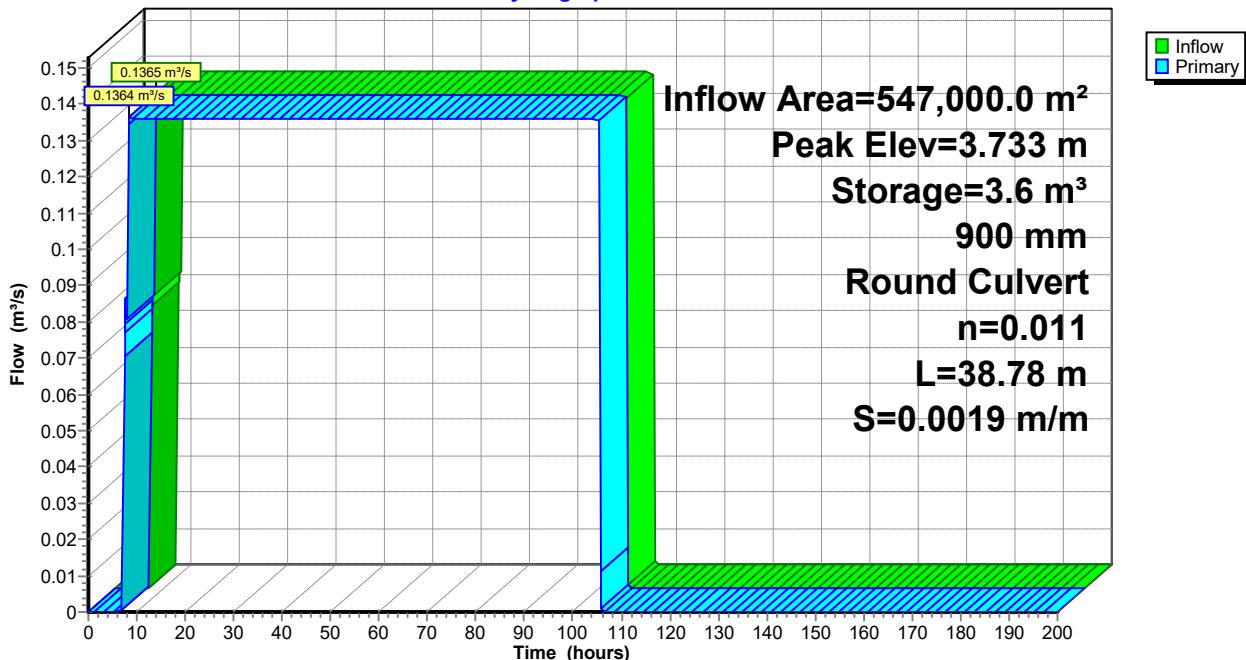
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	900 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.636 m ²

Primary OutFlow Max=0.1364 m³/s @ 8.36 hrs HW=3.733 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.1364 m³/s @ 1.03 m/s)

Pond 93P: 825 pipe

Hydrograph



Summary for Pond 94P: Pond 1

Inflow Area = 512,000.0 m², 4.30% Impervious, Inflow Depth = 140 mm for 20yr Type 1A event
 Inflow = 3.7400 m³/s @ 8.41 hrs, Volume= 71,693.2 m³
 Outflow = 3.6526 m³/s @ 8.53 hrs, Volume= 71,669.3 m³, Atten= 2%, Lag= 7.4 min
 Discarded = 0.0263 m³/s @ 24.41 hrs, Volume= 5,577.0 m³
 Primary = 3.5870 m³/s @ 8.53 hrs, Volume= 46,852.3 m³
 Routed to Pond 91P : Pond 2
 Secondary = 0.0500 m³/s @ 0.00 hrs, Volume= 19,240.0 m³
 Routed to Pond 91P : Pond 2
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Routed to Pond 93P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.300 m Surf.Area= 17,483.7 m² Storage= 3,905.5 m³

Peak Elev= 4.376 m @ 24.41 hrs Surf.Area= 22,178.2 m² Storage= 25,481.2 m³ (21,575.7 m³ above start)

Plug-Flow detention time= 1,069.6 min calculated for 67,763.8 m³ (95% of inflow)

Center-of-Mass det. time= 932.3 min (1,688.3 - 756.0)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0263 m³/s @ 24.41 hrs HW=4.376 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0263 m³/s)

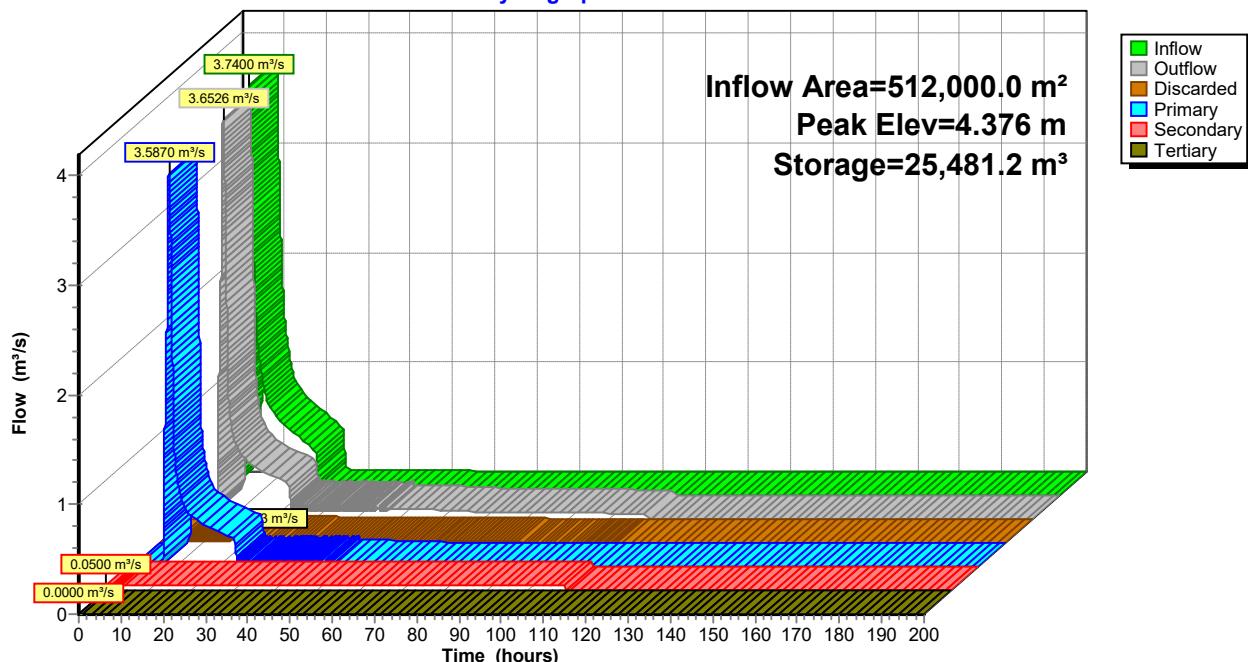
Primary OutFlow Max=3.5868 m³/s @ 8.53 hrs HW=3.855 m TW=3.527 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 3.5868 m³/s @ 0.45 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 0.00 hrs HW=3.300 m TW=3.100 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.300 m TW=2.600 m (Dynamic Tailwater)
 ↗ 4=Culvert (Controls 0.0000 m³/s)
 ↗ 5=Scruffy dome (Controls 0.0000 m³/s)

Pond 94P: Pond 1

Hydrograph



Summary for Pond 96P: Canal (Northern)

Inflow Area = 70,000.0 m², 0.00% Impervious, Inflow Depth = 158 mm for 20yr Type 1A event
 Inflow = 0.6821 m³/s @ 8.20 hrs, Volume= 11,055.5 m³
 Outflow = 0.6142 m³/s @ 8.35 hrs, Volume= 11,032.9 m³, Atten= 10%, Lag= 8.9 min
 Primary = 0.6142 m³/s @ 8.35 hrs, Volume= 11,032.9 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.675 m @ 8.37 hrs Surf.Area= 510.3 m² Storage= 376.5 m³

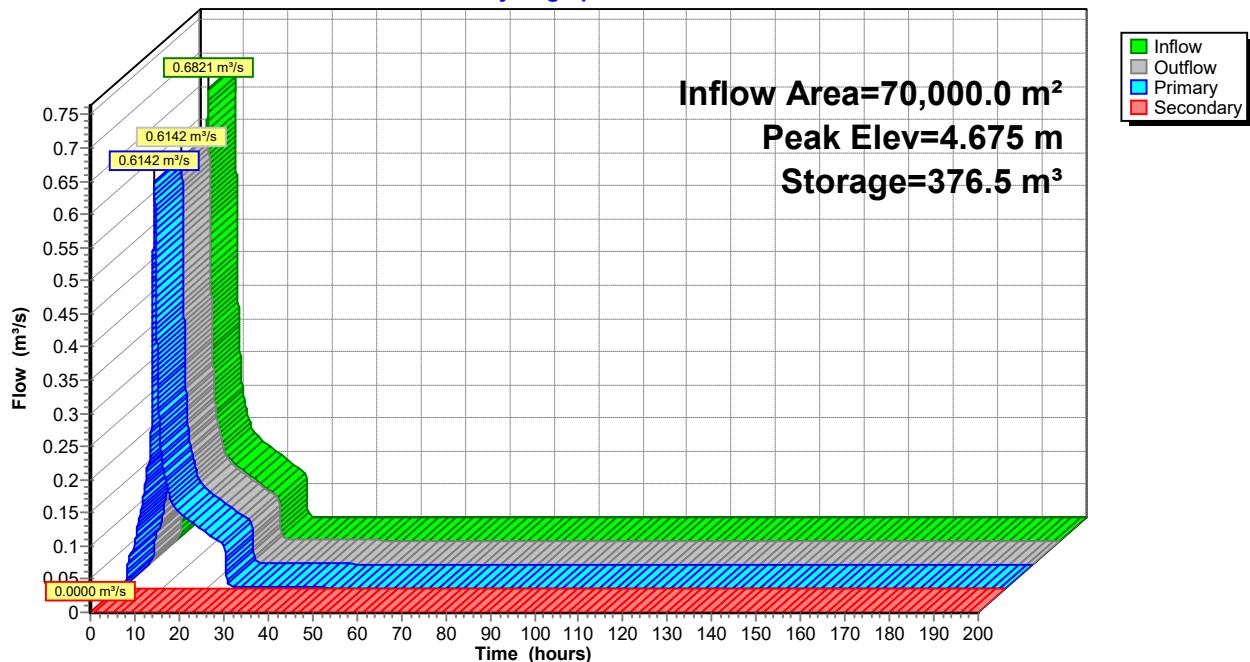
Plug-Flow detention time= 56.0 min calculated for 11,032.4 m³ (100% of inflow)
 Center-of-Mass det. time= 54.9 min (754.2 - 699.2)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	621.4 m ³	2.60 mW x 75.00 mL x 1.50 mH 75m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	690 mm Round Culvert L= 35.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.374 m ²
#2	Secondary	5.000 m	75.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83

Primary OutFlow Max=0.6135 m³/s @ 8.35 hrs HW=4.674 m TW=4.293 m (Dynamic Tailwater)
 ↑1=Culvert (Inlet Controls 0.6135 m³/s @ 1.64 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑2=Overflow (Controls 0.0000 m³/s)

Pond 96P: Canal (Northern)**Hydrograph**

Summary for Pond 62P: Canal (Western) Current

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 190 mm for 50yr Type 1A event
 Inflow = 2.6316 m³/s @ 8.20 hrs, Volume= 42,879.9 m³
 Outflow = 2.3560 m³/s @ 8.37 hrs, Volume= 42,705.1 m³, Atten= 10%, Lag= 10.4 min
 Discarded = 0.0151 m³/s @ 8.37 hrs, Volume= 2,289.5 m³
 Primary = 1.7740 m³/s @ 8.37 hrs, Volume= 39,567.6 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.5669 m³/s @ 8.37 hrs, Volume= 847.9 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.822 m @ 8.37 hrs Surf.Area= 4,091.0 m² Storage= 3,417.9 m³

Plug-Flow detention time= 154.0 min calculated for 42,702.9 m³ (100% of inflow)
 Center-of-Mass det. time= 151.3 min (845.0 - 693.7)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 1,748.1 m ²

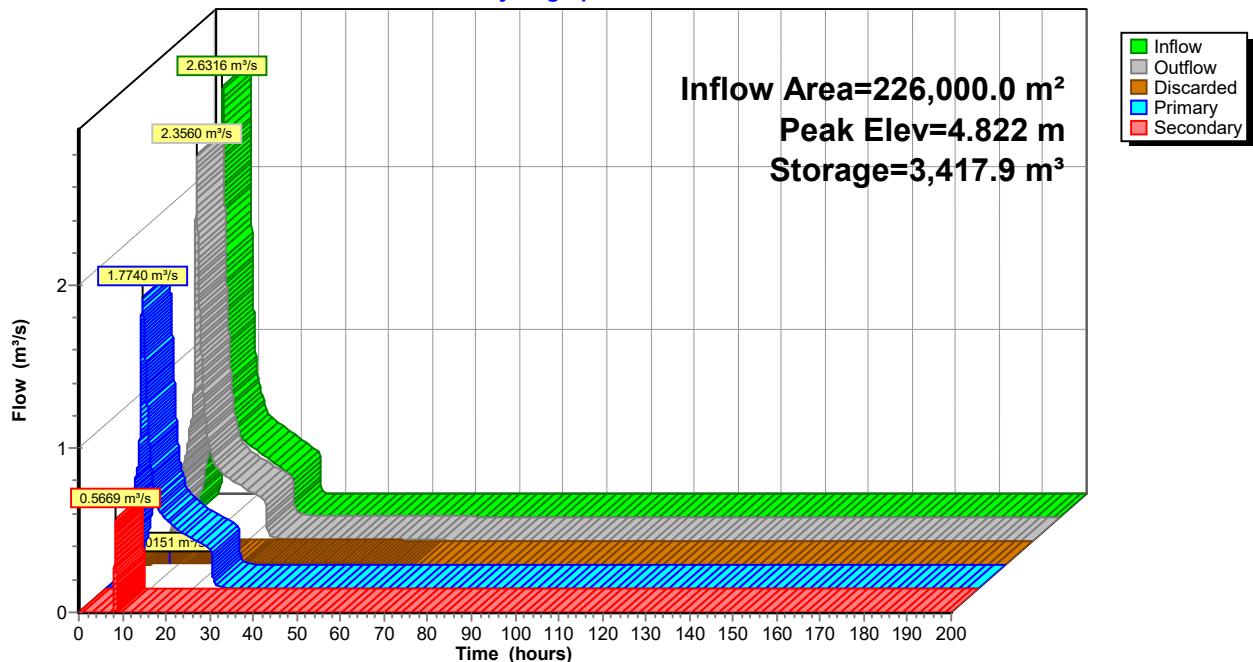
Discarded OutFlow Max=0.0151 m³/s @ 8.37 hrs HW=4.822 m (Free Discharge)
 ↑ 4=Exfiltration (Exfiltration Controls 0.0151 m³/s)

Primary OutFlow Max=1.7740 m³/s @ 8.37 hrs HW=4.822 m TW=4.334 m (Dynamic Tailwater)
 ↑ 1=Culvert (Barrel Controls 1.7740 m³/s @ 1.91 m/s)

Secondary OutFlow Max=0.5668 m³/s @ 8.37 hrs HW=4.822 m (Free Discharge)
 ↑ 2=Spillway (Weir Controls 0.5668 m³/s @ 0.49 m/s)
 3=Overflow (Controls 0.0000 m³/s)

Pond 62P: Canal (Western) Current

Hydrograph



Summary for Pond 91P: Pond 2

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 148 mm for 50yr Type 1A event
 Inflow = 4.4525 m³/s @ 8.51 hrs, Volume= 80,815.8 m³
 Outflow = 0.2165 m³/s @ 24.00 hrs, Volume= 83,130.3 m³, Atten= 95%, Lag= 929.2 min
 Discarded = 0.0804 m³/s @ 24.00 hrs, Volume= 29,777.4 m³
 Primary = 0.1361 m³/s @ 24.00 hrs, Volume= 53,352.9 m³
 Routed to Pond 95P : 525 Pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.100 m Surf.Area= 24,604.8 m² Storage= 5,435.2 m³
 Peak Elev= 4.551 m @ 24.00 hrs Surf.Area= 34,406.2 m² Storage= 50,902.2 m³ (45,466.9 m³ above start)

Plug-Flow detention time= 2,574.9 min calculated for 77,694.1 m³ (96% of inflow)
 Center-of-Mass det. time= 2,149.1 min (3,750.6 - 1,601.5)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	81,468.4 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	474.5	0.0	0.0	474.5
2.800	5,907.2	537.1	537.1	5,907.3
3.000	20,004.5	2,452.2	2,989.2	20,004.8
3.200	29,205.2	4,892.0	7,881.3	29,206.2
3.400	29,965.9	5,916.9	13,798.2	29,976.6
3.600	30,727.5	6,069.2	19,867.4	30,748.2
3.800	31,631.9	6,235.7	26,103.1	31,661.2
4.000	32,382.3	6,401.3	32,504.4	32,422.2
4.200	33,112.6	6,549.4	39,053.7	33,163.7
4.400	33,844.5	6,695.6	45,749.3	33,907.0
4.600	34,590.5	6,843.4	52,592.7	34,664.5
4.800	35,339.0	6,992.8	59,585.5	35,424.6
5.000	36,095.3	7,143.3	66,728.8	36,192.7
5.200	36,849.2	7,294.3	74,023.1	36,958.6
5.400	37,605.2	7,445.3	81,468.4	37,726.9

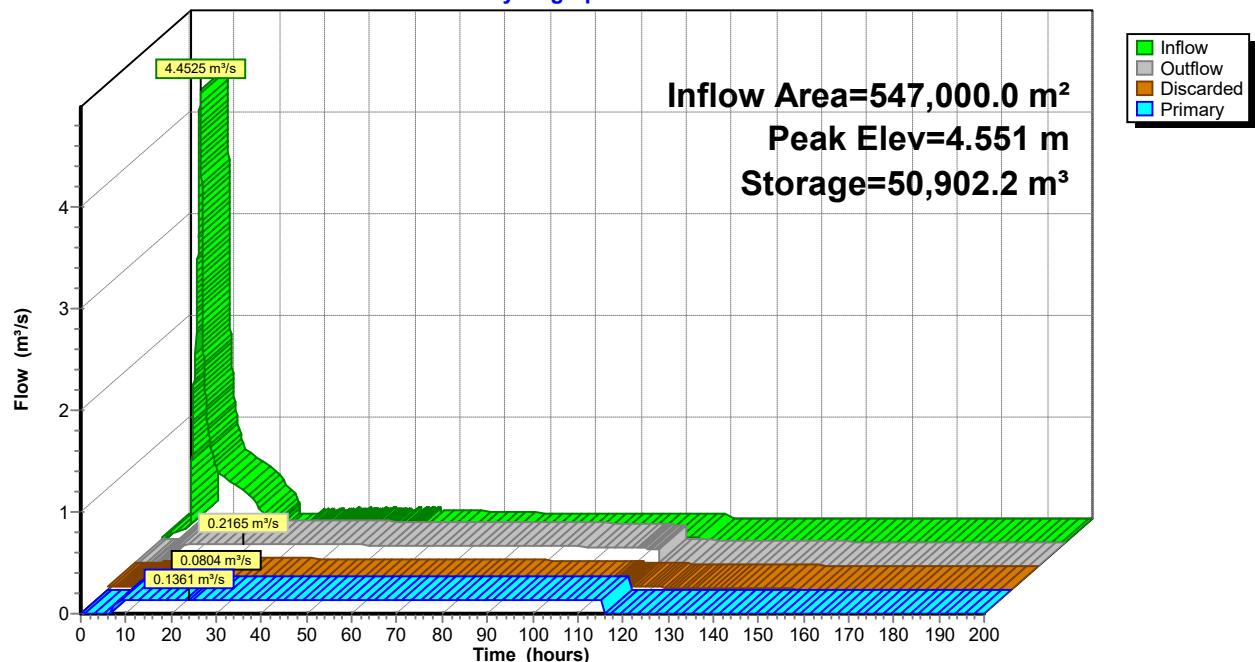
Device	Routing	Invert	Outlet Devices (Turned on 2 times)
#1	Primary	3.240 m	Pump - Total 290m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 4,833.0 4,833.3 Head (meters)= 10.000 0.000 -Loss (meters)= 0.775 0.775 =Lift (meters)= 9.225 -0.775
#2	Primary	3.440 m	Pump - Total 490m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 3,333.0 3,333.7 Head (meters)= 10.000 0.000 -Loss (meters)= 0.389 0.389 =Lift (meters)= 9.611 -0.389
#3	Discarded	3.000 m	20.00 mm/hr Exfiltration over Wetted area above 3.000 m Excluded Wetted area = 20,004.8 m ²

Discarded OutFlow Max=0.0804 m³/s @ 24.00 hrs HW=4.551 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0804 m³/s)

Primary OutFlow Max=0.1361 m³/s @ 24.00 hrs HW=4.551 m TW=4.494 m (Dynamic Tailwater)
 ↗ 1=Pump - Total 290m³/hr (Pump Controls 0.0806 m³/s)
 ↗ 2=Pump - Total 490m³/hr (Pump Controls 0.0556 m³/s)

Pond 91P: Pond 2

Hydrograph



Summary for Pond 92P: Canal (Southern) Current

Inflow Area = 490,000.0 m², 0.00% Impervious, Inflow Depth = 176 mm for 50yr Type 1A event
 Inflow = 4.4409 m³/s @ 8.27 hrs, Volume= 86,278.8 m³
 Outflow = 4.2883 m³/s @ 8.39 hrs, Volume= 85,874.5 m³, Atten= 3%, Lag= 7.3 min
 Discarded = 0.0257 m³/s @ 24.02 hrs, Volume= 4,985.7 m³
 Primary = 4.2691 m³/s @ 8.39 hrs, Volume= 80,888.8 m³
 Routed to Pond 94P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.551 m @ 24.02 hrs Surf.Area= 0.7941 ha Storage= 5.399 MI

Plug-Flow detention time= 174.9 min calculated for 85,870.2 m³ (100% of inflow)
 Center-of-Mass det. time= 160.6 min (893.4 - 732.8)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	10.479 MI	2.60 mW x 1,315.00 mL x 1.50 mH 1315m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 0.3962 ha

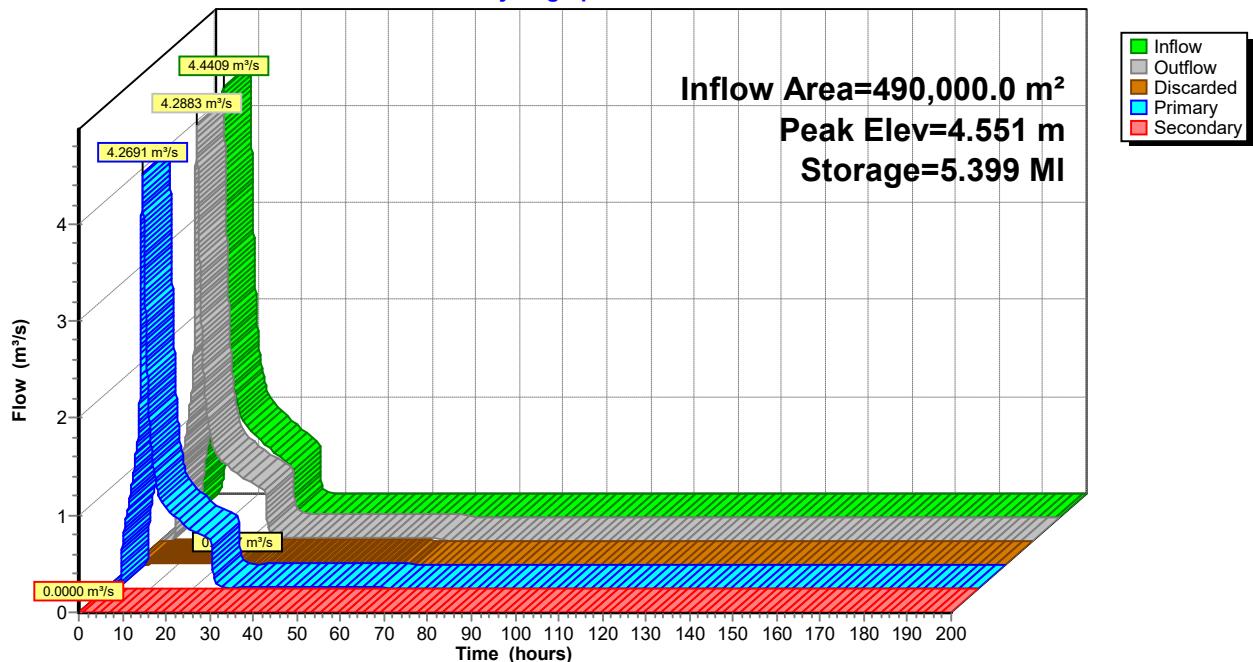
Discarded OutFlow Max=0.0257 m³/s @ 24.02 hrs HW=4.551 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0257 m³/s)

Primary OutFlow Max=4.2690 m³/s @ 8.39 hrs HW=4.334 m TW=3.865 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 4.2690 m³/s @ 1.12 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Pond 92P: Canal (Southern) Current

Hydrograph



Summary for Pond 93P: 825 pipe

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 106 mm for 50yr Type 1A event
 Inflow = 0.4088 m³/s @ 24.01 hrs, Volume= 58,024.6 m³
 Outflow = 0.4088 m³/s @ 24.01 hrs, Volume= 58,022.1 m³, Atten= 0%, Lag= 0.0 min
 Primary = 0.4088 m³/s @ 24.01 hrs, Volume= 58,022.1 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.986 m @ 24.01 hrs Surf.Area= 3.1 m² Storage= 4.4 m³

Plug-Flow detention time= 0.5 min calculated for 58,022.1 m³ (100% of inflow)
 Center-of-Mass det. time= 0.3 min (3,522.9 - 3,522.7)

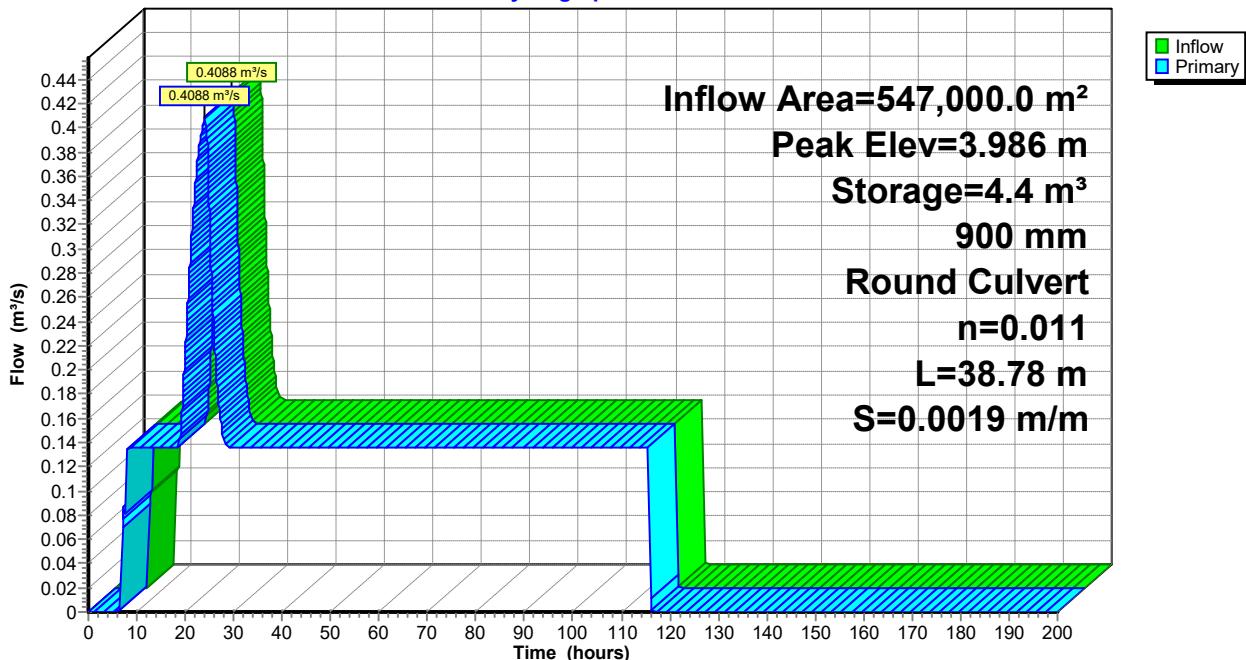
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	900 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.636 m ²

Primary OutFlow Max=0.4088 m³/s @ 24.01 hrs HW=3.986 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.4088 m³/s @ 1.38 m/s)

Pond 93P: 825 pipe

Hydrograph



Summary for Pond 94P: Pond 1

Inflow Area = 512,000.0 m², 4.30% Impervious, Inflow Depth = 167 mm for 50yr Type 1A event
 Inflow = 4.3823 m³/s @ 8.38 hrs, Volume= 85,330.7 m³
 Outflow = 4.2818 m³/s @ 8.49 hrs, Volume= 85,292.4 m³, Atten= 2%, Lag= 6.7 min
 Discarded = 0.0296 m³/s @ 24.01 hrs, Volume= 6,868.0 m³
 Primary = 4.2160 m³/s @ 8.49 hrs, Volume= 52,509.7 m³
 Routed to Pond 91P : Pond 2
 Secondary = 0.0500 m³/s @ 0.00 hrs, Volume= 21,239.4 m³
 Routed to Pond 91P : Pond 2
 Tertiary = 0.2727 m³/s @ 24.01 hrs, Volume= 4,675.3 m³
 Routed to Pond 93P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.300 m Surf.Area= 17,483.7 m² Storage= 3,905.5 m³

Peak Elev= 4.551 m @ 24.01 hrs Surf.Area= 22,774.2 m² Storage= 29,418.1 m³ (25,512.6 m³ above start)

Plug-Flow detention time= 1,139.9 min calculated for 81,382.8 m³ (95% of inflow)

Center-of-Mass det. time= 1,004.4 min (1,768.9 - 764.5)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0296 m³/s @ 24.01 hrs HW=4.551 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0296 m³/s)

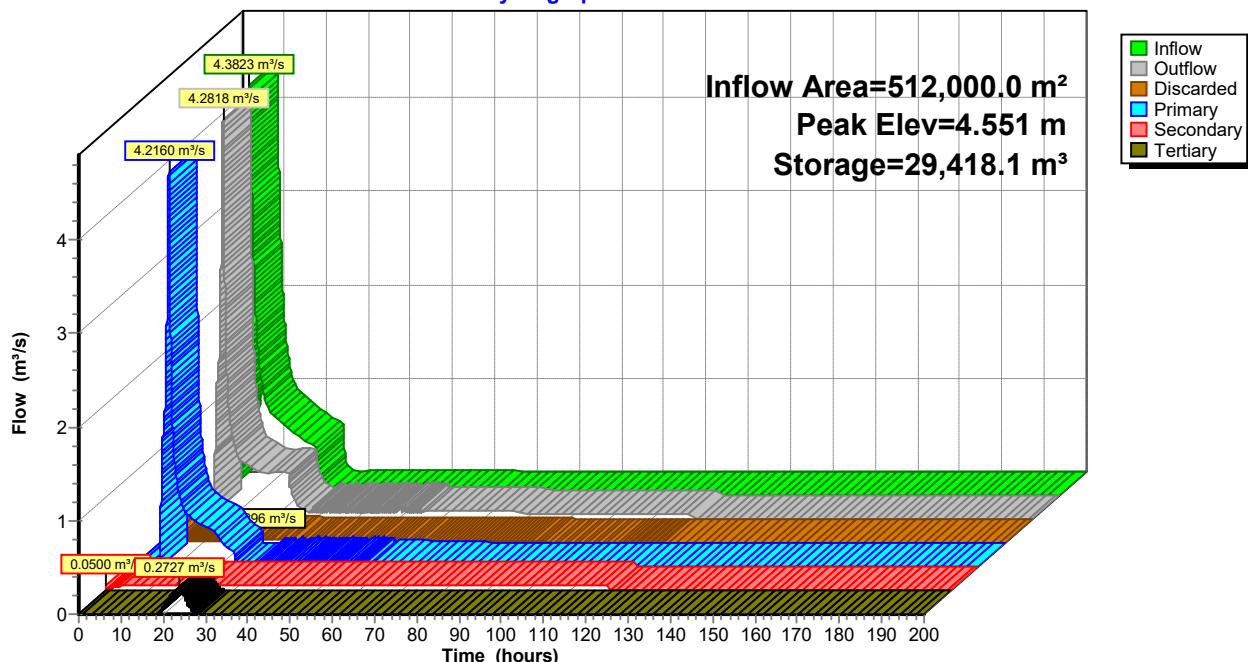
Primary OutFlow Max=4.2157 m³/s @ 8.49 hrs HW=3.867 m TW=3.684 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 4.2157 m³/s @ 0.48 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 0.00 hrs HW=3.300 m TW=3.100 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.2727 m³/s @ 24.01 hrs HW=4.551 m TW=3.986 m (Dynamic Tailwater)
 ↗ 4=Culvert (Passes 0.2727 m³/s of 0.9919 m³/s potential flow)
 ↗ 5=Scruffy dome (Weir Controls 0.2727 m³/s @ 0.51 m/s)

Pond 94P: Pond 1

Hydrograph



Summary for Pond 96P: Canal (Northern)

Inflow Area = 70,000.0 m², 0.00% Impervious, Inflow Depth = 190 mm for 50yr Type 1A event
 Inflow = 0.8151 m³/s @ 8.20 hrs, Volume= 13,281.4 m³
 Outflow = 0.7195 m³/s @ 8.38 hrs, Volume= 13,258.3 m³, Atten= 12%, Lag= 10.8 min
 Primary = 0.7195 m³/s @ 8.38 hrs, Volume= 13,258.3 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

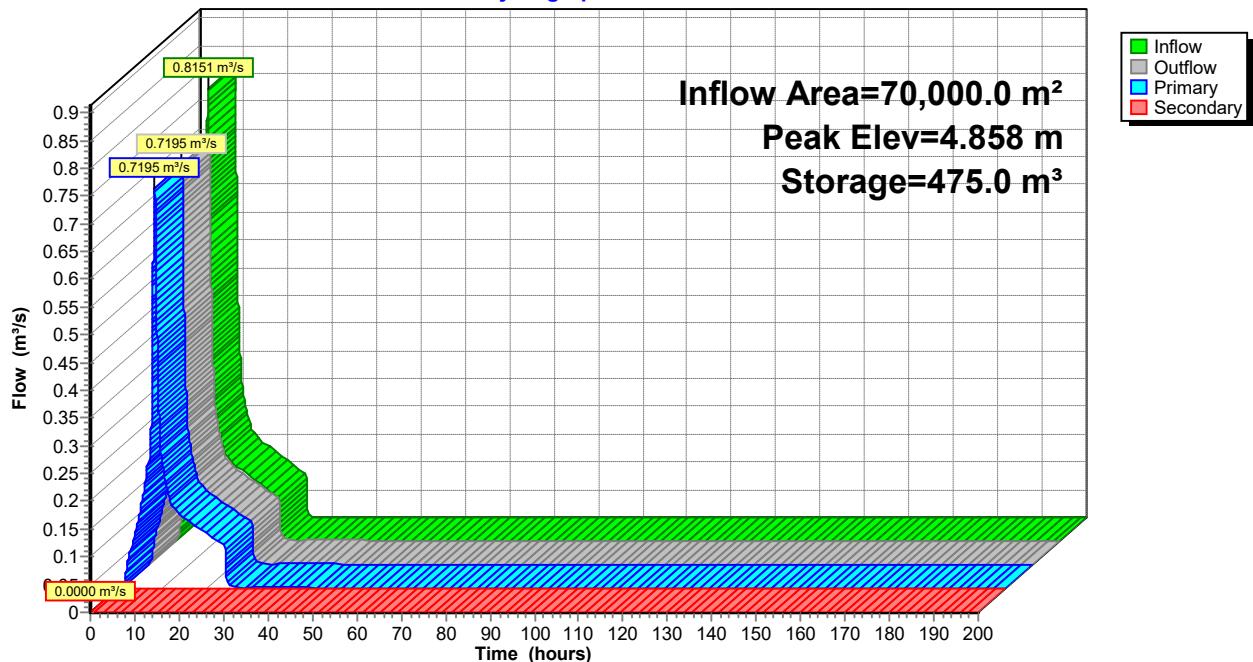
Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.858 m @ 8.38 hrs Surf.Area= 567.0 m² Storage= 475.0 m³

Plug-Flow detention time= 63.9 min calculated for 13,258.3 m³ (100% of inflow)
 Center-of-Mass det. time= 62.6 min (756.2 - 693.7)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	621.4 m ³	2.60 mW x 75.00 mL x 1.50 mH 75m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	690 mm Round Culvert L= 35.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.374 m ²
#2	Secondary	5.000 m	75.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83

Primary OutFlow Max=0.7193 m³/s @ 8.38 hrs HW=4.858 m TW=4.334 m (Dynamic Tailwater)
 ↑1=Culvert (Inlet Controls 0.7193 m³/s @ 1.92 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑2=Overflow (Controls 0.0000 m³/s)

Pond 96P: Canal (Northern)**Hydrograph**

Summary for Pond 62P: Canal (Western) Current

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 215 mm for 100yr Type 1A event
 Inflow = 2.9656 m³/s @ 8.20 hrs, Volume= 48,502.6 m³
 Outflow = 2.7578 m³/s @ 8.33 hrs, Volume= 48,327.2 m³, Atten= 7%, Lag= 8.1 min
 Discarded = 0.0156 m³/s @ 8.33 hrs, Volume= 2,424.2 m³
 Primary = 1.8570 m³/s @ 8.33 hrs, Volume= 44,346.1 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.8852 m³/s @ 8.33 hrs, Volume= 1,557.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.860 m @ 8.33 hrs Surf.Area= 4,171.7 m² Storage= 3,574.4 m³

Plug-Flow detention time= 146.2 min calculated for 48,324.8 m³ (100% of inflow)
 Center-of-Mass det. time= 143.9 min (834.1 - 690.2)

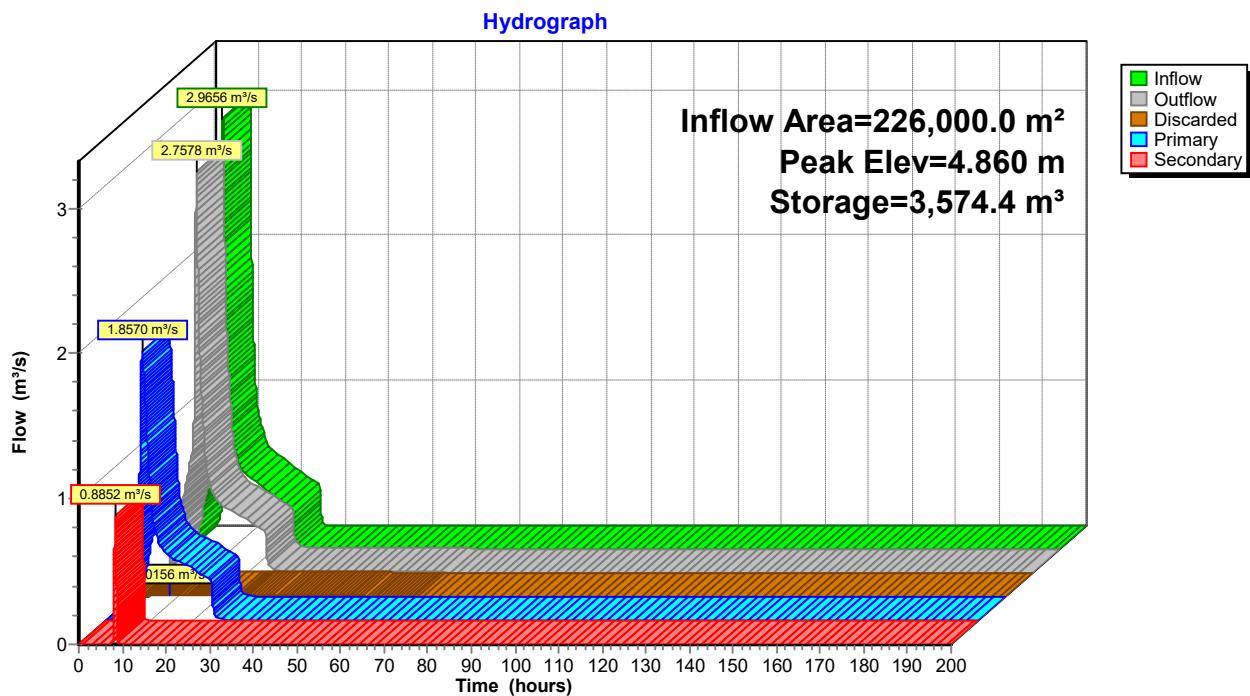
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 1,748.1 m ²

Discarded OutFlow Max=0.0156 m³/s @ 8.33 hrs HW=4.860 m (Free Discharge)
 ↑ 4=Exfiltration (Exfiltration Controls 0.0156 m³/s)

Primary OutFlow Max=1.8569 m³/s @ 8.33 hrs HW=4.860 m TW=4.359 m (Dynamic Tailwater)
 ↑ 1=Culvert (Barrel Controls 1.8569 m³/s @ 1.94 m/s)

Secondary OutFlow Max=0.8850 m³/s @ 8.33 hrs HW=4.860 m (Free Discharge)
 ↑ 2=Spillway (Weir Controls 0.8850 m³/s @ 0.58 m/s)
 3=Overflow (Controls 0.0000 m³/s)

Pond 62P: Canal (Western) Current

Summary for Pond 91P: Pond 2

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 150 mm for 100yr Type 1A event
 Inflow = 4.8349 m³/s @ 8.38 hrs, Volume= 82,293.3 m³
 Outflow = 0.2174 m³/s @ 21.57 hrs, Volume= 84,600.5 m³, Atten= 96%, Lag= 791.1 min
 Discarded = 0.0813 m³/s @ 21.57 hrs, Volume= 30,360.8 m³
 Primary = 0.1361 m³/s @ 21.57 hrs, Volume= 54,239.7 m³
 Routed to Pond 95P : 525 Pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.100 m Surf.Area= 24,604.8 m² Storage= 5,435.2 m³

Peak Elev= 4.594 m @ 21.57 hrs Surf.Area= 34,567.2 m² Storage= 52,378.5 m³ (46,943.2 m³ above start)

Plug-Flow detention time= 2,630.5 min calculated for 79,160.4 m³ (96% of inflow)

Center-of-Mass det. time= 2,204.8 min (3,776.3 - 1,571.6)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	81,468.4 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	474.5	0.0	0.0	474.5
2.800	5,907.2	537.1	537.1	5,907.3
3.000	20,004.5	2,452.2	2,989.2	20,004.8
3.200	29,205.2	4,892.0	7,881.3	29,206.2
3.400	29,965.9	5,916.9	13,798.2	29,976.6
3.600	30,727.5	6,069.2	19,867.4	30,748.2
3.800	31,631.9	6,235.7	26,103.1	31,661.2
4.000	32,382.3	6,401.3	32,504.4	32,422.2
4.200	33,112.6	6,549.4	39,053.7	33,163.7
4.400	33,844.5	6,695.6	45,749.3	33,907.0
4.600	34,590.5	6,843.4	52,592.7	34,664.5
4.800	35,339.0	6,992.8	59,585.5	35,424.6
5.000	36,095.3	7,143.3	66,728.8	36,192.7
5.200	36,849.2	7,294.3	74,023.1	36,958.6
5.400	37,605.2	7,445.3	81,468.4	37,726.9

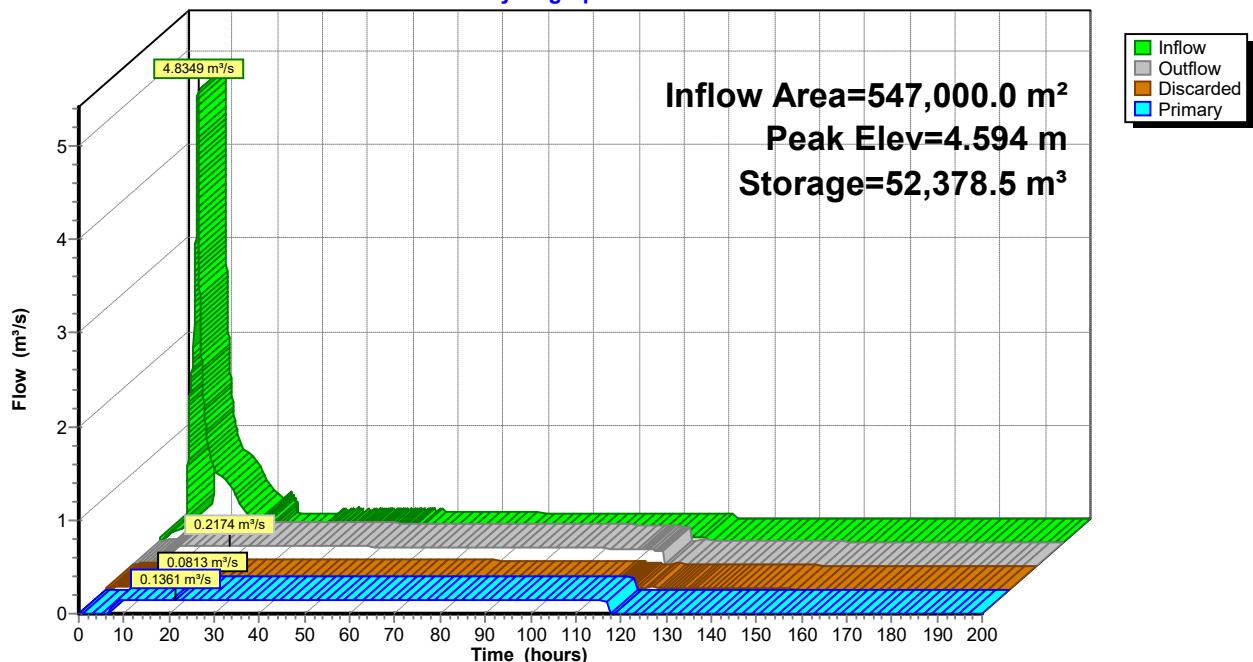
Device	Routing	Invert	Outlet Devices (Turned on 2 times)
#1	Primary	3.240 m	Pump - Total 290m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 4,833.0 4,833.3 Head (meters)= 10.000 0.000 -Loss (meters)= 0.775 0.775 =Lift (meters)= 9.225 -0.775
#2	Primary	3.440 m	Pump - Total 490m3/hr Discharges@4.000 m Turns Off<3.100 m 200 mm Diam. x 25.00 m Long Discharge, Hazen-Williams C= 130 Flow (l/min)= 3,333.0 3,333.7 Head (meters)= 10.000 0.000 -Loss (meters)= 0.389 0.389 =Lift (meters)= 9.611 -0.389
#3	Discarded	3.000 m	20.00 mm/hr Exfiltration over Wetted area above 3.000 m Excluded Wetted area = 20,004.8 m ²

Discarded OutFlow Max=0.0813 m³/s @ 21.57 hrs HW=4.594 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0813 m³/s)

Primary OutFlow Max=0.1361 m³/s @ 21.57 hrs HW=4.594 m TW=4.494 m (Dynamic Tailwater)
 ↗ 1=Pump - Total 290m³/hr (Pump Controls 0.0806 m³/s)
 ↗ 2=Pump - Total 490m³/hr (Pump Controls 0.0556 m³/s)

Pond 91P: Pond 2

Hydrograph



Summary for Pond 92P: Canal (Southern) Current

Inflow Area = 490,000.0 m², 0.00% Impervious, Inflow Depth = 199 mm for 100yr Type 1A event
 Inflow = 4.8918 m³/s @ 8.25 hrs, Volume= 97,433.6 m³
 Outflow = 4.7294 m³/s @ 8.37 hrs, Volume= 97,027.4 m³, Atten= 3%, Lag= 7.5 min
 Discarded = 0.0270 m³/s @ 21.56 hrs, Volume= 5,278.1 m³
 Primary = 4.7095 m³/s @ 8.37 hrs, Volume= 91,749.4 m³
 Routed to Pond 94P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.594 m @ 21.56 hrs Surf.Area= 0.8148 ha Storage= 5.748 MI

Plug-Flow detention time= 166.4 min calculated for 97,027.4 m³ (100% of inflow)
 Center-of-Mass det. time= 153.0 min (883.7 - 730.7)

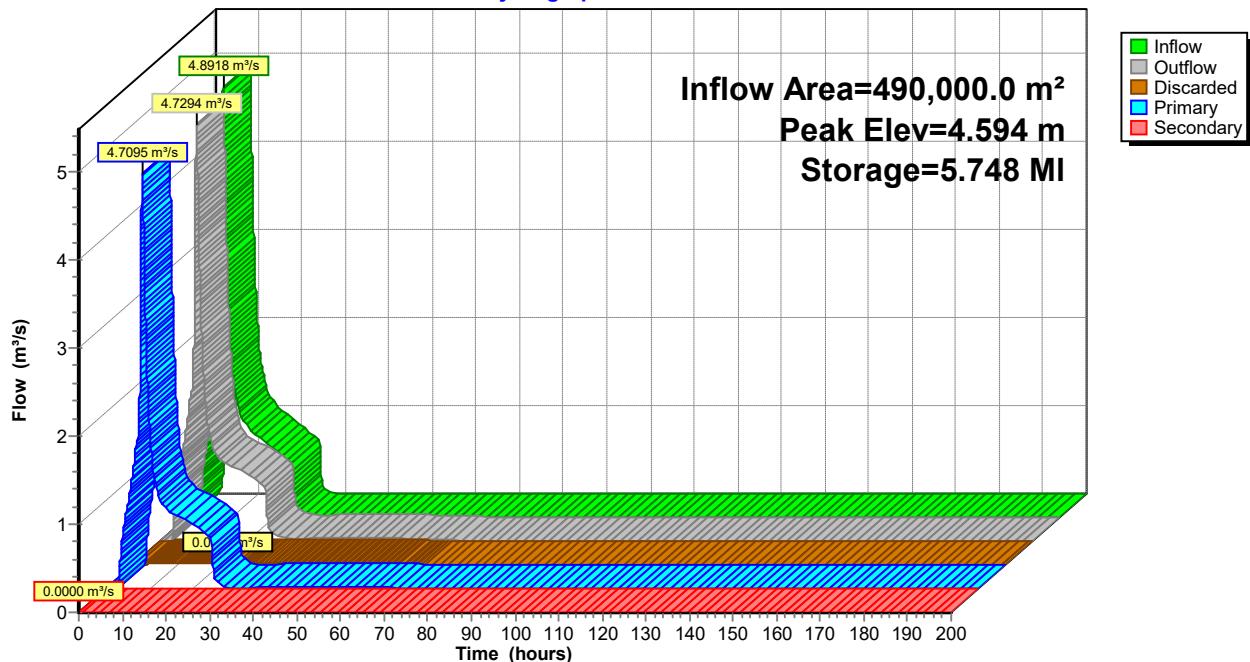
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	10.479 MI	2.60 mW x 1,315.00 mL x 1.50 mH 1315m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.700 m	20.00 mm/hr Exfiltration over Wetted area above 3.700 m Excluded Wetted area = 0.3962 ha

Discarded OutFlow Max=0.0270 m³/s @ 21.56 hrs HW=4.594 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0270 m³/s)

Primary OutFlow Max=4.7092 m³/s @ 8.37 hrs HW=4.360 m TW=3.873 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 4.7092 m³/s @ 1.16 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.600 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Pond 92P: Canal (Southern) Current**Hydrograph**

Summary for Pond 93P: 825 pipe

Inflow Area = 547,000.0 m², 10.42% Impervious, Inflow Depth = 127 mm for 100yr Type 1A event

Inflow = 0.6549 m³/s @ 21.58 hrs, Volume= 69,440.1 m³

Outflow = 0.6549 m³/s @ 21.58 hrs, Volume= 69,437.5 m³, Atten= 0%, Lag= 0.0 min

Primary = 0.6549 m³/s @ 21.58 hrs, Volume= 69,437.5 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Peak Elev= 4.167 m @ 21.58 hrs Surf.Area= 3.1 m² Storage= 4.9 m³

Plug-Flow detention time= 0.4 min calculated for 69,434.0 m³ (100% of inflow)

Center-of-Mass det. time= 0.2 min (3,201.9 - 3,201.7)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

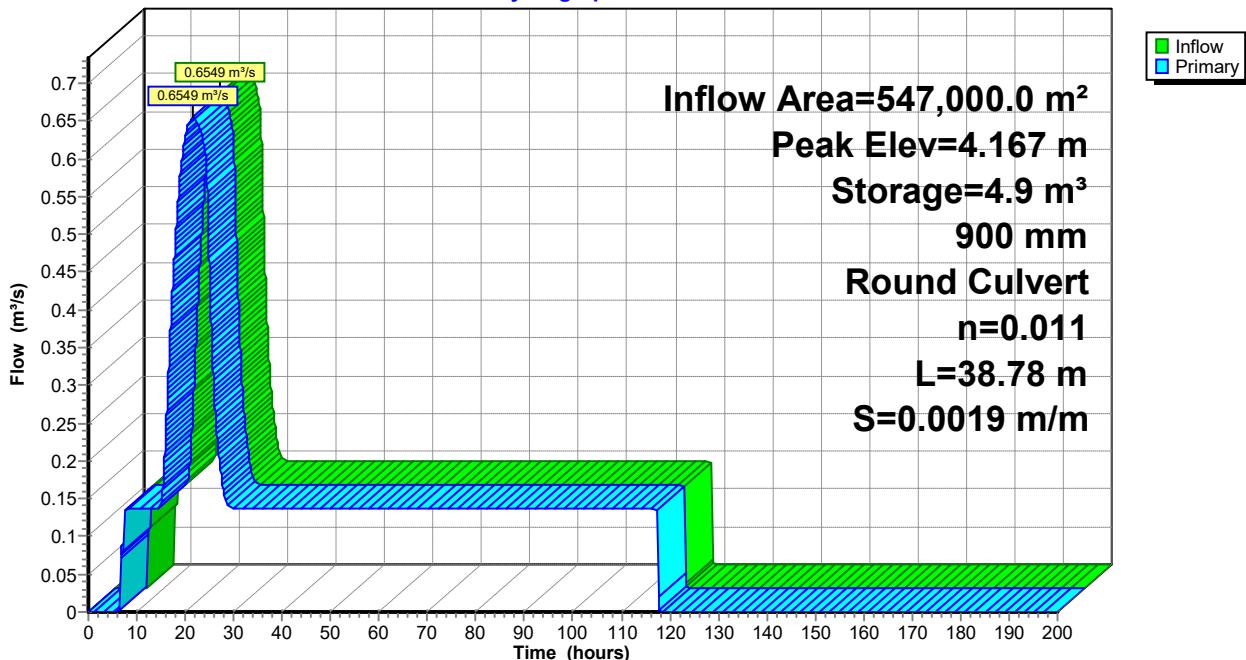
Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	900 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.636 m ²

Primary OutFlow Max=0.6549 m³/s @ 21.58 hrs HW=4.167 m (Free Discharge)

↑—1=Culvert (Barrel Controls 0.6549 m³/s @ 1.57 m/s)

Pond 93P: 825 pipe

Hydrograph



Summary for Pond 94P: Pond 1

Inflow Area = 512,000.0 m², 4.30% Impervious, Inflow Depth = 189 mm for 100yr Type 1A event
 Inflow = 4.8386 m³/s @ 8.37 hrs, Volume= 96,741.0 m³
 Outflow = 4.6483 m³/s @ 8.38 hrs, Volume= 96,699.7 m³, Atten= 4%, Lag= 1.0 min
 Discarded = 0.0304 m³/s @ 21.58 hrs, Volume= 7,143.8 m³
 Primary = 4.5823 m³/s @ 8.38 hrs, Volume= 52,803.9 m³
 Routed to Pond 91P : Pond 2
 Secondary = 0.0500 m³/s @ 0.00 hrs, Volume= 21,548.1 m³
 Routed to Pond 91P : Pond 2
 Tertiary = 0.5188 m³/s @ 21.58 hrs, Volume= 15,203.8 m³
 Routed to Pond 93P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs

Starting Elev= 3.300 m Surf.Area= 17,483.7 m² Storage= 3,905.5 m³

Peak Elev= 4.594 m @ 21.58 hrs Surf.Area= 22,921.3 m² Storage= 30,393.2 m³ (26,487.8 m³ above start)

Plug-Flow detention time= 1,049.5 min calculated for 92,789.6 m³ (96% of inflow)

Center-of-Mass det. time= 931.1 min (1,695.7 - 764.7)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0304 m³/s @ 21.58 hrs HW=4.594 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0304 m³/s)

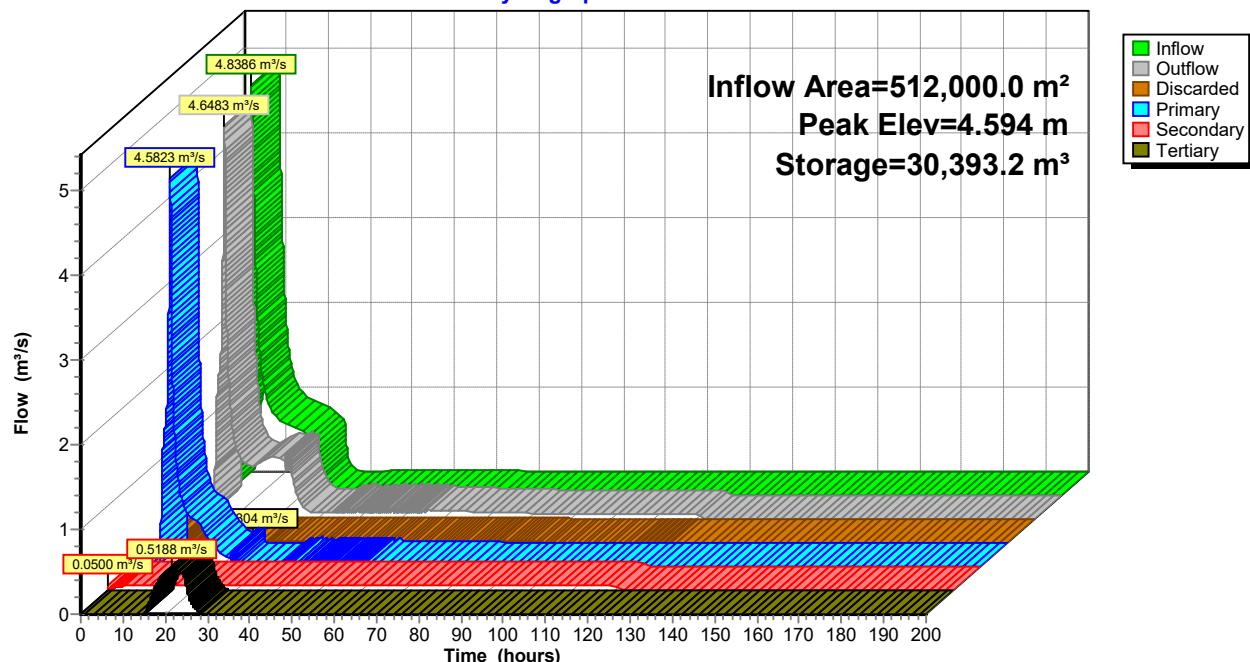
Primary OutFlow Max=4.5540 m³/s @ 8.38 hrs HW=3.873 m TW=3.760 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 4.5540 m³/s @ 0.49 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 0.00 hrs HW=3.300 m TW=3.100 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.5188 m³/s @ 21.58 hrs HW=4.594 m TW=4.167 m (Dynamic Tailwater)
 ↗ 4=Culvert (Passes 0.5188 m³/s of 0.9276 m³/s potential flow)
 ↗ 5=Scruffy dome (Weir Controls 0.5188 m³/s @ 0.64 m/s)

Pond 94P: Pond 1

Hydrograph



Summary for Pond 96P: Canal (Northern)

Inflow Area = 70,000.0 m², 0.00% Impervious, Inflow Depth = 215 mm for 100yr Type 1A event
 Inflow = 0.9186 m³/s @ 8.20 hrs, Volume= 15,022.9 m³
 Outflow = 0.8135 m³/s @ 8.38 hrs, Volume= 14,999.8 m³, Atten= 11%, Lag= 10.9 min
 Primary = 0.7966 m³/s @ 8.38 hrs, Volume= 14,997.5 m³
 Routed to Pond 92P : Canal (Southern) Current
 Secondary = 0.0169 m³/s @ 8.38 hrs, Volume= 2.3 m³

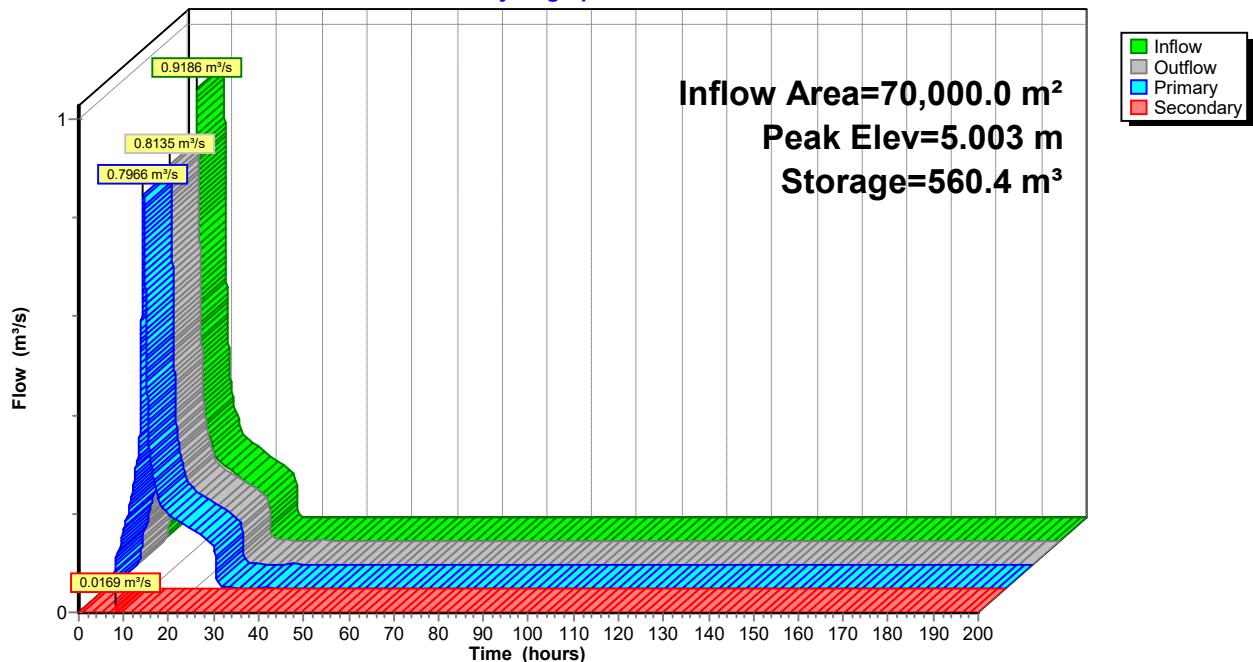
Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 5.003 m @ 8.38 hrs Surf.Area= 612.4 m² Storage= 560.4 m³

Plug-Flow detention time= 61.1 min calculated for 14,999.8 m³ (100% of inflow)
 Center-of-Mass det. time= 59.9 min (750.1 - 690.2)

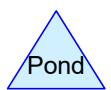
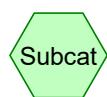
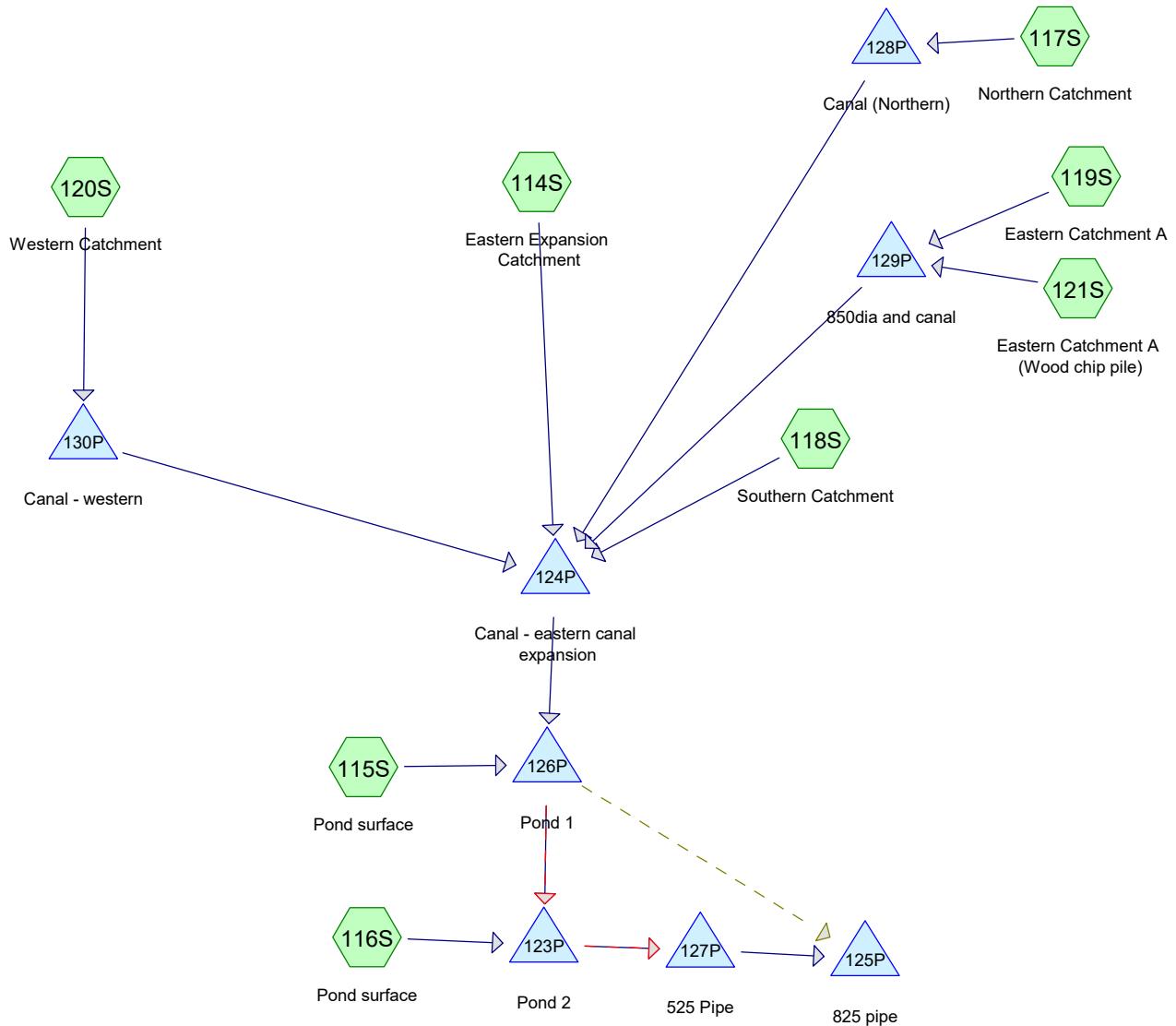
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	621.4 m ³	2.60 mW x 75.00 mL x 1.50 mH 75m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	690 mm Round Culvert L= 35.00 m Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.374 m ²
#2	Secondary	5.000 m	75.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83

Primary OutFlow Max=0.7966 m³/s @ 8.38 hrs HW=5.003 m TW=4.360 m (Dynamic Tailwater)
 ↑1=Culvert (Inlet Controls 0.7966 m³/s @ 2.13 m/s)

Secondary OutFlow Max=0.0168 m³/s @ 8.38 hrs HW=5.003 m (Free Discharge)
 ↑2=Overflow (Weir Controls 0.0168 m³/s @ 0.08 m/s)

Pond 96P: Canal (Northern)**Hydrograph**

Full expansion 67.3Ha



Routing Diagram for 12377 - SW Pond - revised (expansion model) R3
 Prepared by Hawthorn Geddes Eng & Arch Ltd, Printed 10/08/2022
 HydroCAD® 10.20-2d s/n 05482 © 2021 HydroCAD Software Solutions LLC

Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 106 mm for 5yr Type 1A event
 Inflow = 3.9622 m³/s @ 8.25 hrs, Volume= 71,417.5 m³
 Outflow = 3.6806 m³/s @ 8.43 hrs, Volume= 71,389.2 m³, Atten= 7%, Lag= 10.9 min
 Discarded = 0.0214 m³/s @ 24.39 hrs, Volume= 3,174.3 m³
 Primary = 3.6603 m³/s @ 8.43 hrs, Volume= 68,215.0 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.319 m @ 24.39 hrs Surf.Area= 10,310.6 m² Storage= 5,559.3 m³ (4,045.4 m³ above start)

Plug-Flow detention time= 163.2 min calculated for 69,871.8 m³ (98% of inflow)
 Center-of-Mass det. time= 124.1 min (861.1 - 737.0)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

Discarded OutFlow Max=0.0214 m³/s @ 24.39 hrs HW=4.319 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0214 m³/s)

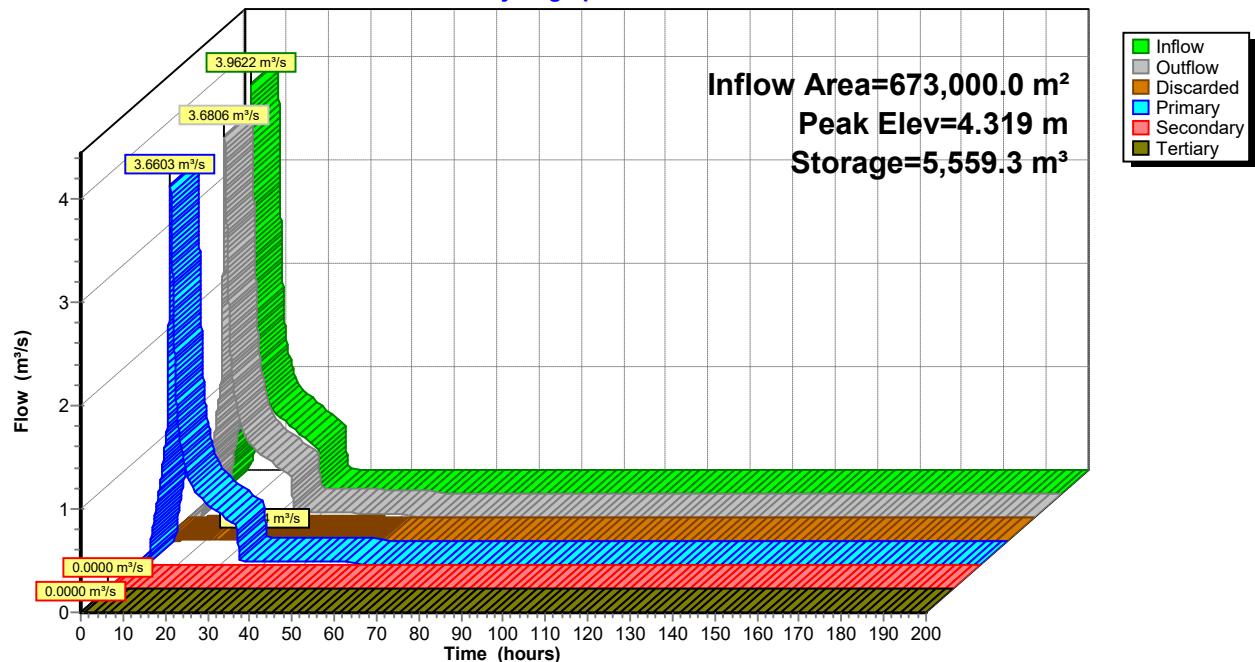
Primary OutFlow Max=3.6601 m³/s @ 8.43 hrs HW=4.297 m TW=3.853 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 3.6601 m³/s @ 1.06 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 4=Proposed Spillway Overflow (Controls 0.0000 m³/s)

Pond 124P: Canal - eastern canal expansion

Hydrograph



Summary for Pond 126P: Pond 1

Inflow Area = 695,000.0 m², 3.17% Impervious, Inflow Depth = 102 mm for 5yr Type 1A event
 Inflow = 3.7269 m³/s @ 8.43 hrs, Volume= 70,920.6 m³
 Outflow = 3.6394 m³/s @ 8.55 hrs, Volume= 70,909.9 m³, Atten= 2%, Lag= 7.3 min
 Discarded = 0.0252 m³/s @ 24.41 hrs, Volume= 5,275.5 m³
 Primary = 3.5738 m³/s @ 8.55 hrs, Volume= 46,890.9 m³
 Routed to Pond 123P : Pond 2
 Secondary = 0.0500 m³/s @ 3.28 hrs, Volume= 18,743.5 m³
 Routed to Pond 123P : Pond 2
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Routed to Pond 125P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.300 m Surf.Area= 17,483.6 m² Storage= 3,905.5 m³
 Peak Elev= 4.319 m @ 24.41 hrs Surf.Area= 21,984.1 m² Storage= 24,230.1 m³ (20,324.6 m³ above start)

Plug-Flow detention time= 1,026.9 min calculated for 67,004.5 m³ (94% of inflow)
 Center-of-Mass det. time= 880.0 min (1,659.1 - 779.1)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0252 m³/s @ 24.41 hrs HW=4.319 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0252 m³/s)

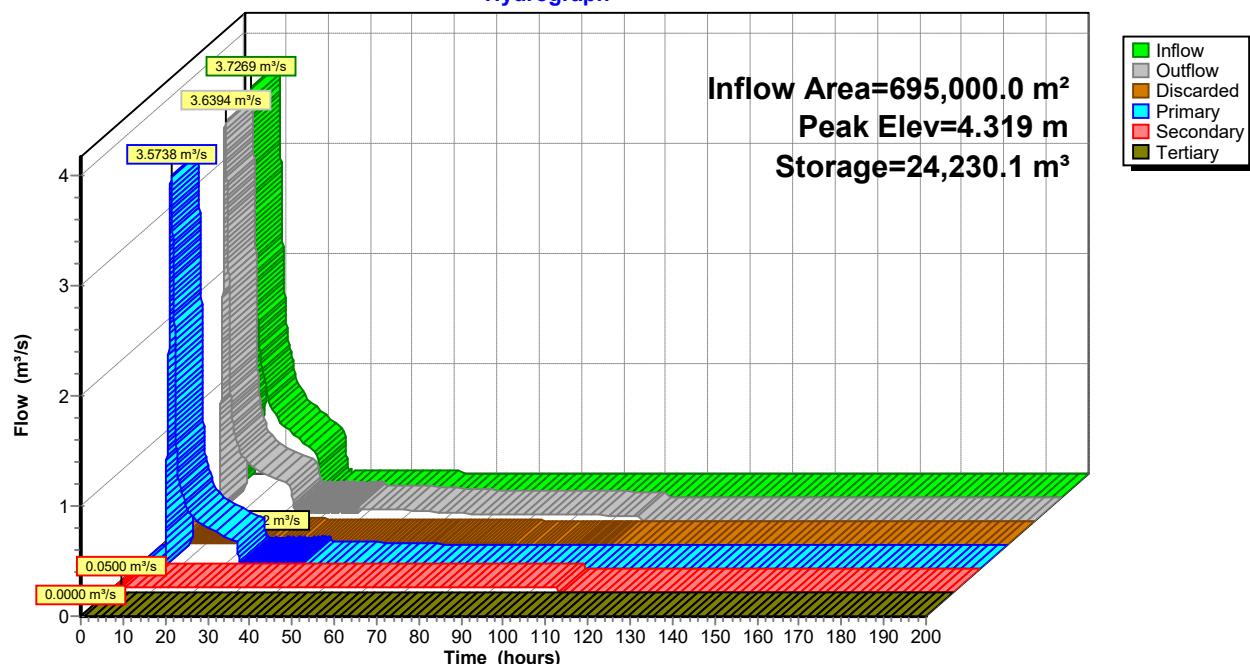
Primary OutFlow Max=3.5737 m³/s @ 8.55 hrs HW=3.855 m TW=3.476 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 3.5737 m³/s @ 0.45 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 3.28 hrs HW=3.300 m TW=3.105 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.300 m TW=2.600 m (Dynamic Tailwater)
 ↗ 4=Culvert (Controls 0.0000 m³/s)
 ↗ 5=Scruffy dome (Controls 0.0000 m³/s)

Pond 126P: Pond 1

Hydrograph



Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 128 mm for 10yr Type 1A event
 Inflow = 4.7361 m³/s @ 8.25 hrs, Volume= 85,947.9 m³
 Outflow = 4.4204 m³/s @ 8.43 hrs, Volume= 85,909.2 m³, Atten= 7%, Lag= 10.5 min
 Discarded = 0.0305 m³/s @ 24.31 hrs, Volume= 4,859.9 m³
 Primary = 4.3980 m³/s @ 8.43 hrs, Volume= 81,049.3 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.519 m @ 24.31 hrs Surf.Area= 11,747.5 m² Storage= 7,767.6 m³ (6,253.6 m³ above start)

Plug-Flow detention time= 211.2 min calculated for 84,391.1 m³ (98% of inflow)
 Center-of-Mass det. time= 168.3 min (906.4 - 738.1)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

Discarded OutFlow Max=0.0305 m³/s @ 24.31 hrs HW=4.519 m (Free Discharge)
 ↗ 3=Exfiltration (Exfiltration Controls 0.0305 m³/s)

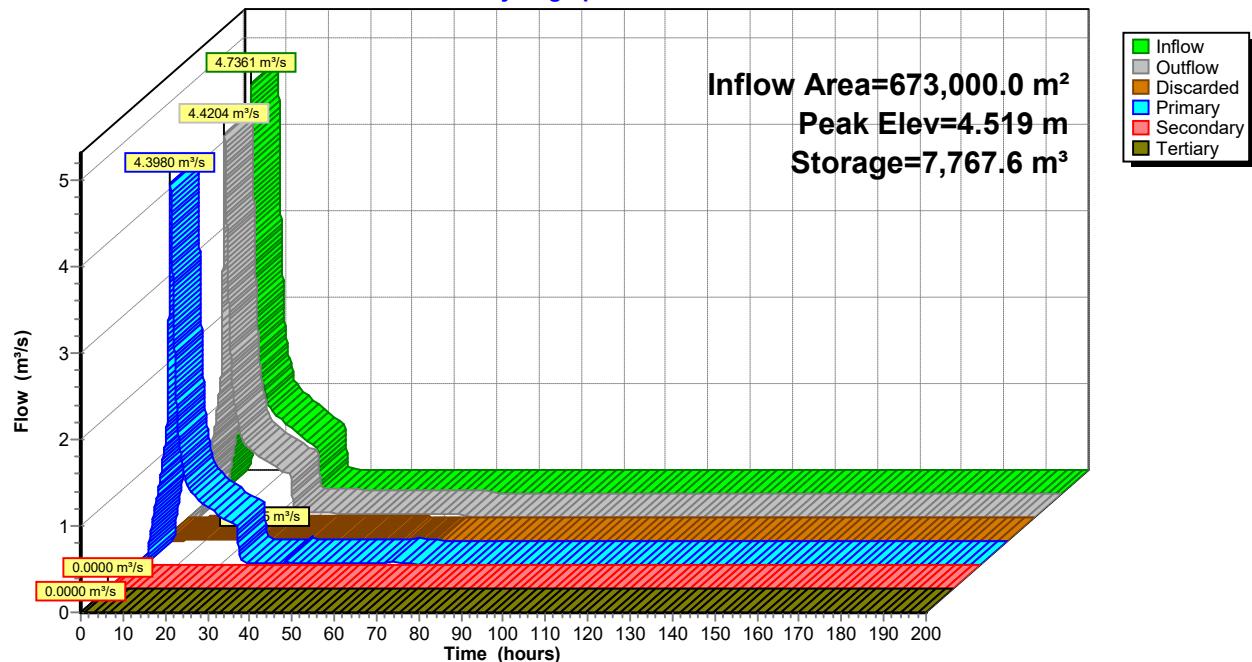
Primary OutFlow Max=4.3979 m³/s @ 8.43 hrs HW=4.342 m TW=3.867 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 4.3979 m³/s @ 1.13 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↗ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↗ 4=Proposed Spillway Overflow (Controls 0.0000 m³/s)

Pond 124P: Canal - eastern canal expansion

Hydrograph



Summary for Pond 126P: Pond 1

Inflow Area = 695,000.0 m², 3.17% Impervious, Inflow Depth = 121 mm for 10yr Type 1A event
 Inflow = 4.4770 m³/s @ 8.42 hrs, Volume= 84,260.2 m³
 Outflow = 4.3857 m³/s @ 8.54 hrs, Volume= 84,214.2 m³, Atten= 2%, Lag= 6.8 min
 Discarded = 0.0290 m³/s @ 24.32 hrs, Volume= 6,847.2 m³
 Primary = 4.3198 m³/s @ 8.54 hrs, Volume= 54,680.4 m³
 Routed to Pond 123P : Pond 2
 Secondary = 0.0500 m³/s @ 2.91 hrs, Volume= 21,298.5 m³
 Routed to Pond 123P : Pond 2
 Tertiary = 0.1286 m³/s @ 24.32 hrs, Volume= 1,388.1 m³
 Routed to Pond 125P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.300 m Surf.Area= 17,483.6 m² Storage= 3,905.5 m³
 Peak Elev= 4.519 m @ 24.32 hrs Surf.Area= 22,666.0 m² Storage= 28,700.2 m³ (24,794.8 m³ above start)

Plug-Flow detention time= 1,173.7 min calculated for 80,304.7 m³ (95% of inflow)
 Center-of-Mass det. time= 1,008.5 min (1,810.5 - 802.0)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0290 m³/s @ 24.32 hrs HW=4.519 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0290 m³/s)

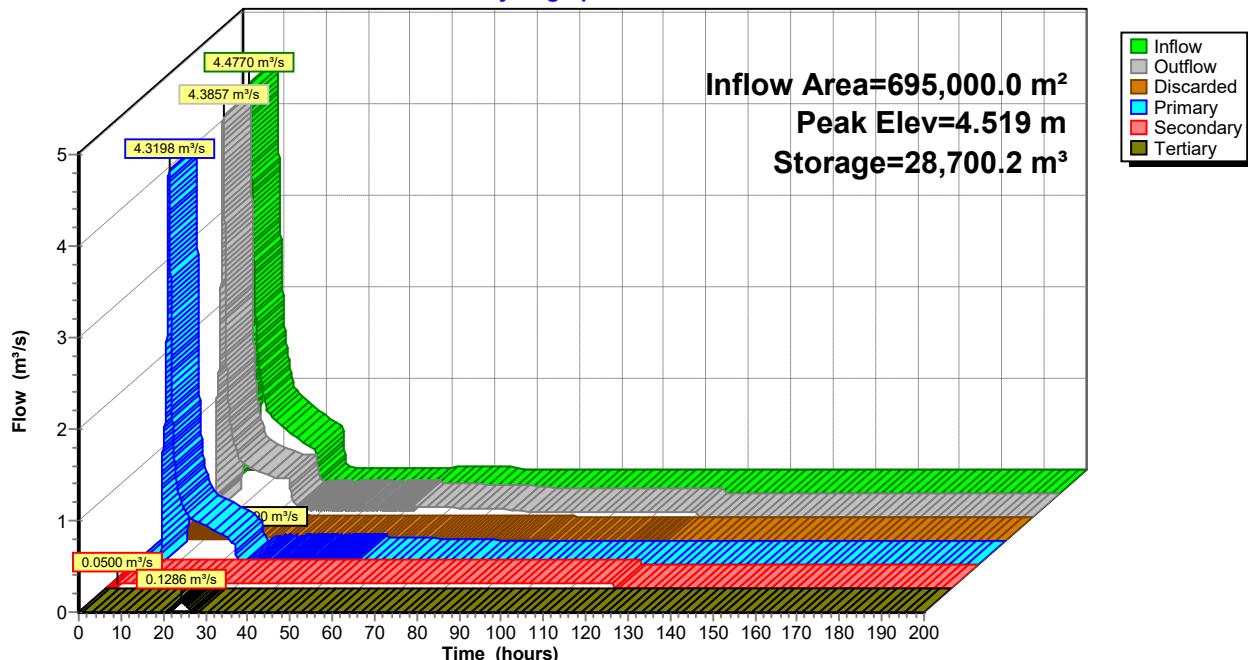
Primary OutFlow Max=4.3196 m³/s @ 8.54 hrs HW=3.869 m TW=3.640 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 4.3196 m³/s @ 0.49 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 2.91 hrs HW=3.300 m TW=3.107 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.1286 m³/s @ 24.32 hrs HW=4.519 m TW=3.881 m (Dynamic Tailwater)
 ↗ 4=Culvert (Passes 0.1286 m³/s of 0.9783 m³/s potential flow)
 ↗ 5=Scruffy dome (Weir Controls 0.1286 m³/s @ 0.40 m/s)

Pond 126P: Pond 1

Hydrograph



Summary for Pond 130P: Canal - western

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 134 mm for 10yr Type 1A event
 Inflow = 1.8783 m³/s @ 8.20 hrs, Volume= 30,315.2 m³
 Outflow = 1.4597 m³/s @ 8.48 hrs, Volume= 30,305.4 m³, Atten= 22%, Lag= 16.9 min
 Discarded = 0.0111 m³/s @ 8.48 hrs, Volume= 1,485.7 m³
 Primary = 1.4486 m³/s @ 8.48 hrs, Volume= 28,819.7 m³
 Routed to Pond 124P : Canal - eastern canal expansion
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 2,033.1 m² Storage= 442.6 m³
 Peak Elev= 4.676 m @ 8.48 hrs Surf.Area= 3,779.1 m² Storage= 2,841.0 m³ (2,398.4 m³ above start)

Plug-Flow detention time= 168.0 min calculated for 29,861.3 m³ (99% of inflow)
 Center-of-Mass det. time= 147.3 min (851.9 - 704.6)

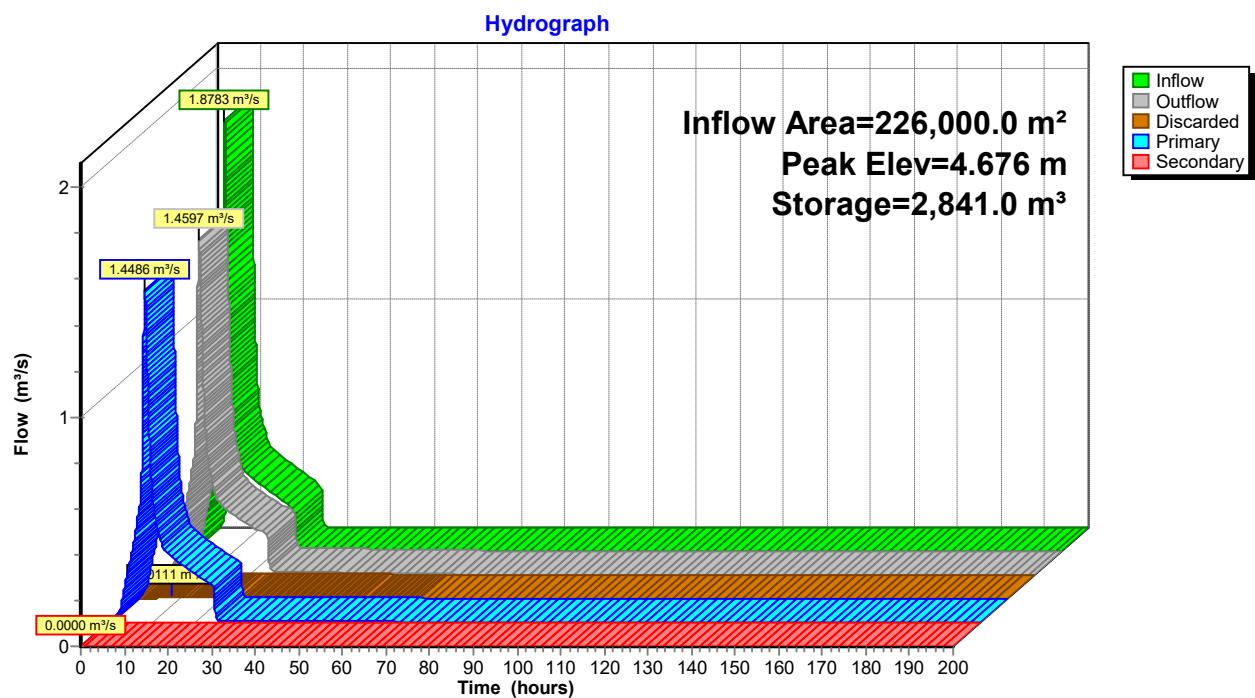
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 2,108.8 m ²

Discarded OutFlow Max=0.0111 m³/s @ 8.48 hrs HW=4.676 m (Free Discharge)
 ↗ 4=Exfiltration (Exfiltration Controls 0.0111 m³/s)

Primary OutFlow Max=1.4486 m³/s @ 8.48 hrs HW=4.676 m TW=4.340 m (Dynamic Tailwater)
 ↗ 1=Culvert (Barrel Controls 1.4486 m³/s @ 1.79 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↗ 2=Spillway (Controls 0.0000 m³/s)
 ↗ 3=Overflow (Controls 0.0000 m³/s)

Pond 130P: Canal - western



Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 151 mm for 20yr Type 1A event
 Inflow = 5.5381 m³/s @ 8.25 hrs, Volume= 101,332.8 m³
 Outflow = 5.1743 m³/s @ 8.42 hrs, Volume= 101,291.6 m³, Atten= 7%, Lag= 10.1 min
 Discarded = 0.0338 m³/s @ 21.97 hrs, Volume= 5,515.8 m³
 Primary = 5.1499 m³/s @ 8.42 hrs, Volume= 95,775.9 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.591 m @ 21.97 hrs Surf.Area= 12,262.4 m² Storage= 8,628.4 m³ (7,114.5 m³ above start)

Plug-Flow detention time= 205.3 min calculated for 99,777.7 m³ (98% of inflow)
 Center-of-Mass det. time= 166.5 min (900.7 - 734.3)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

Discarded OutFlow Max=0.0338 m³/s @ 21.97 hrs HW=4.591 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0338 m³/s)

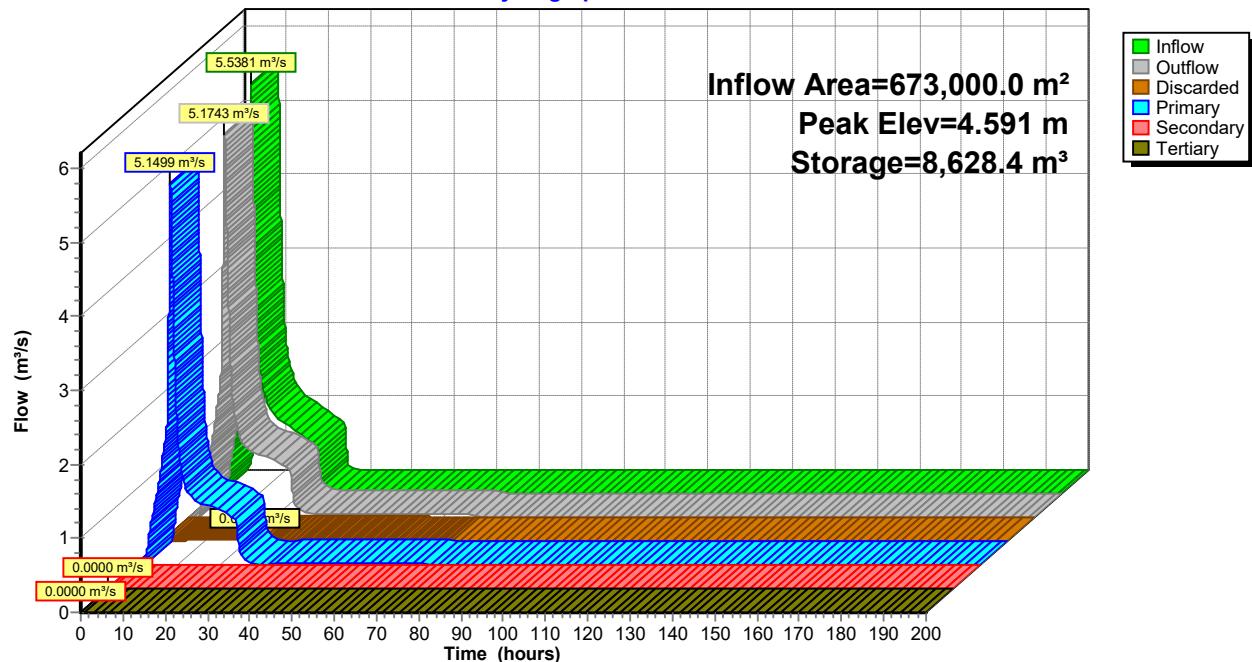
Primary OutFlow Max=5.1497 m³/s @ 8.42 hrs HW=4.386 m TW=3.880 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 5.1497 m³/s @ 1.19 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 4=Proposed Spillway Overflow (Controls 0.0000 m³/s)

Pond 124P: Canal - eastern canal expansion

Hydrograph



Summary for Pond 125P: 825 pipe

Inflow Area = 730,000.0 m², 7.81% Impervious, Inflow Depth = 95 mm for 20yr Type 1A event
 Inflow = 0.6327 m³/s @ 21.98 hrs, Volume= 69,557.6 m³
 Outflow = 0.6327 m³/s @ 21.99 hrs, Volume= 69,555.0 m³, Atten= 0%, Lag= 0.1 min
 Primary = 0.6327 m³/s @ 21.99 hrs, Volume= 69,555.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.192 m @ 21.99 hrs Surf.Area= 3.1 m² Storage= 5.0 m³

Plug-Flow detention time= 0.4 min calculated for 69,551.5 m³ (100% of inflow)
 Center-of-Mass det. time= 0.2 min (3,288.6 - 3,288.4)

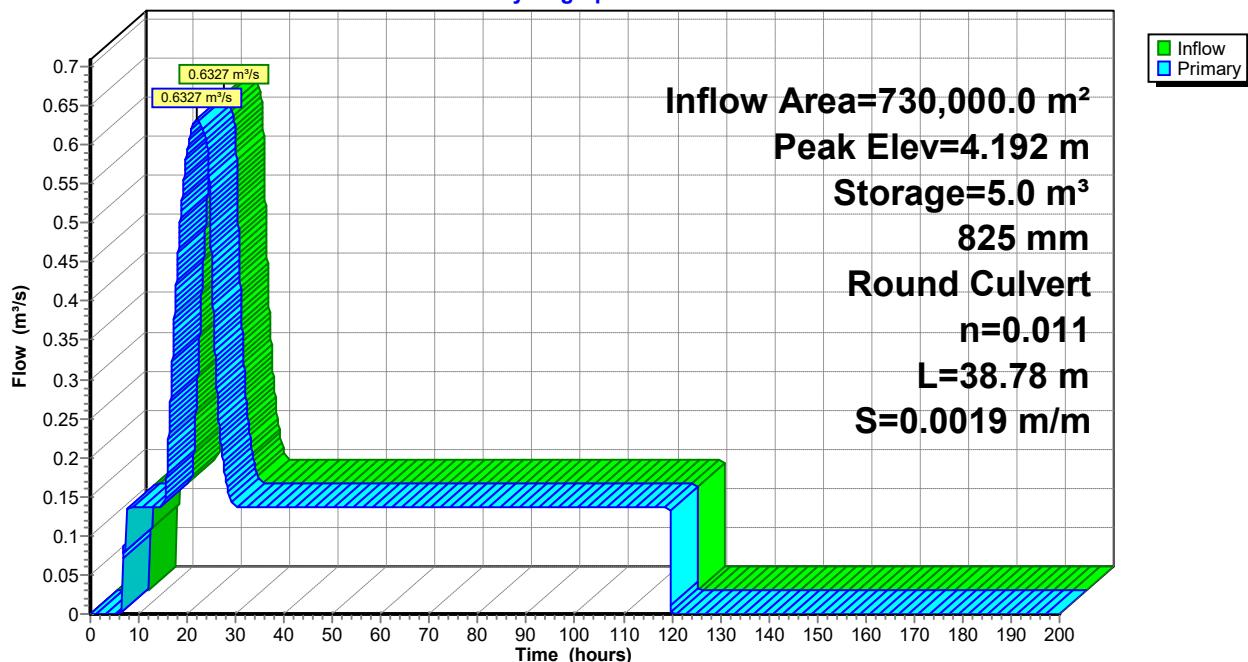
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=0.6327 m³/s @ 21.99 hrs HW=4.192 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.6327 m³/s @ 1.58 m/s)

Pond 125P: 825 pipe

Hydrograph



Summary for Pond 130P: Canal - western

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 158 mm for 20yr Type 1A event
 Inflow = 2.2021 m³/s @ 8.20 hrs, Volume= 35,693.5 m³
 Outflow = 1.8283 m³/s @ 8.43 hrs, Volume= 35,683.0 m³, Atten= 17%, Lag= 14.0 min
 Discarded = 0.0122 m³/s @ 8.43 hrs, Volume= 1,683.9 m³
 Primary = 1.6337 m³/s @ 8.43 hrs, Volume= 33,812.3 m³
 Routed to Pond 124P : Canal - eastern canal expansion
 Secondary = 0.1824 m³/s @ 8.43 hrs, Volume= 186.8 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 2,033.1 m² Storage= 442.6 m³
 Peak Elev= 4.759 m @ 8.43 hrs Surf.Area= 3,956.2 m² Storage= 3,163.1 m³ (2,720.5 m³ above start)

Plug-Flow detention time= 163.4 min calculated for 35,238.6 m³ (99% of inflow)
 Center-of-Mass det. time= 145.6 min (844.9 - 699.2)

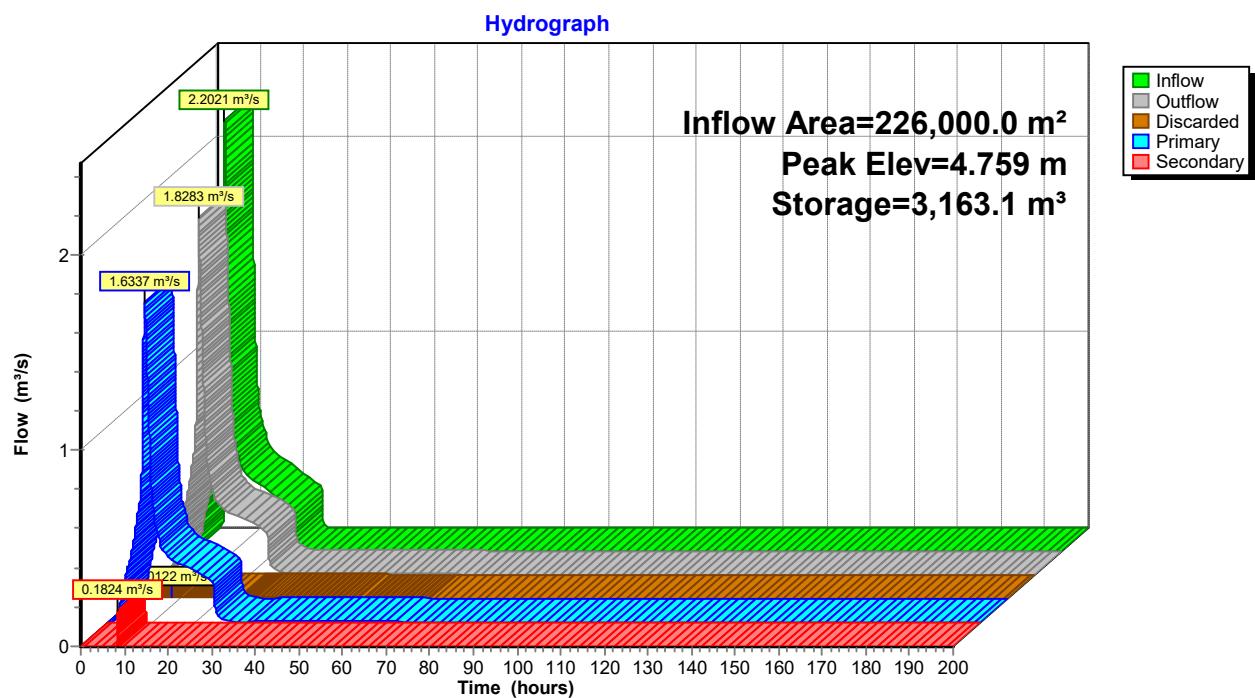
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 2,108.8 m ²

Discarded OutFlow Max=0.0122 m³/s @ 8.43 hrs HW=4.759 m (Free Discharge)
 ↗ 4=Exfiltration (Exfiltration Controls 0.0122 m³/s)

Primary OutFlow Max=1.6337 m³/s @ 8.43 hrs HW=4.759 m TW=4.386 m (Dynamic Tailwater)
 ↗ 1=Culvert (Barrel Controls 1.6337 m³/s @ 1.86 m/s)

Secondary OutFlow Max=0.1823 m³/s @ 8.43 hrs HW=4.759 m (Free Discharge)
 ↗ 2=Spillway (Weir Controls 0.1823 m³/s @ 0.33 m/s)
 ↗ 3=Overflow (Controls 0.0000 m³/s)

Pond 130P: Canal - western



Summary for Pond 125P: 825 pipe

Inflow Area = 730,000.0 m², 7.81% Impervious, Inflow Depth = 122 mm for 50yr Type 1A event
 Inflow = 1.4372 m³/s @ 21.96 hrs, Volume= 88,847.0 m³
 Outflow = 1.4630 m³/s @ 21.94 hrs, Volume= 88,844.4 m³, Atten= 0%, Lag= 0.0 min
 Primary = 1.4630 m³/s @ 21.94 hrs, Volume= 88,844.4 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 5.032 m @ 21.94 hrs Surf.Area= 3.1 m² Storage= 7.6 m³

Plug-Flow detention time= (not calculated: outflow precedes inflow)
 Center-of-Mass det. time= 0.2 min (2,947.5 - 2,947.3)

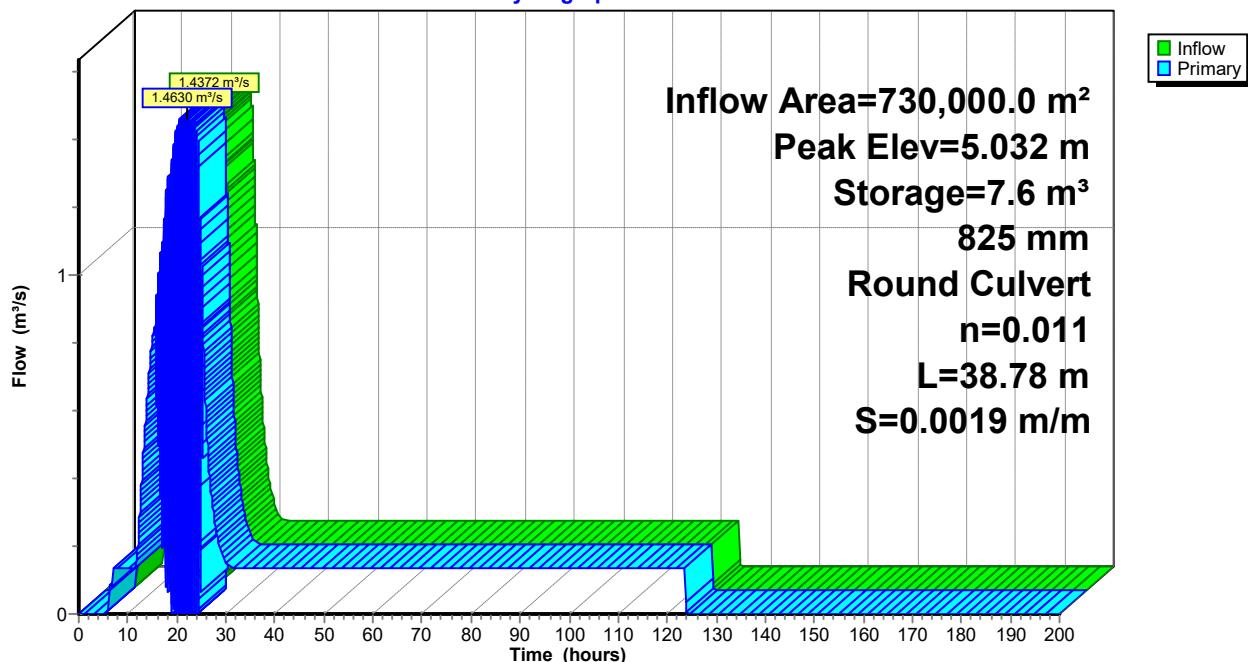
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=1.4341 m³/s @ 21.94 hrs HW=4.998 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 1.4341 m³/s @ 2.68 m/s)

Pond 125P: 825 pipe

Hydrograph



Summary for Pond 130P: Canal - western

Inflow Area = 226,000.0 m², 0.00% Impervious, Inflow Depth = 215 mm for 100yr Type 1A event
 Inflow = 2.9656 m³/s @ 8.20 hrs, Volume= 48,502.6 m³
 Outflow = 2.7630 m³/s @ 8.33 hrs, Volume= 48,490.5 m³, Atten= 7%, Lag= 8.0 min
 Discarded = 0.0136 m³/s @ 8.33 hrs, Volume= 2,052.7 m³
 Primary = 1.8583 m³/s @ 8.33 hrs, Volume= 38,770.3 m³
 Routed to Pond 124P : Canal - eastern canal expansion
 Secondary = 0.8910 m³/s @ 8.33 hrs, Volume= 7,667.4 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 2,033.1 m² Storage= 442.6 m³
 Peak Elev= 4.861 m @ 8.33 hrs Surf.Area= 4,173.0 m² Storage= 3,577.0 m³ (3,134.4 m³ above start)

Plug-Flow detention time= 149.3 min calculated for 48,045.4 m³ (99% of inflow)
 Center-of-Mass det. time= 136.2 min (826.4 - 690.2)

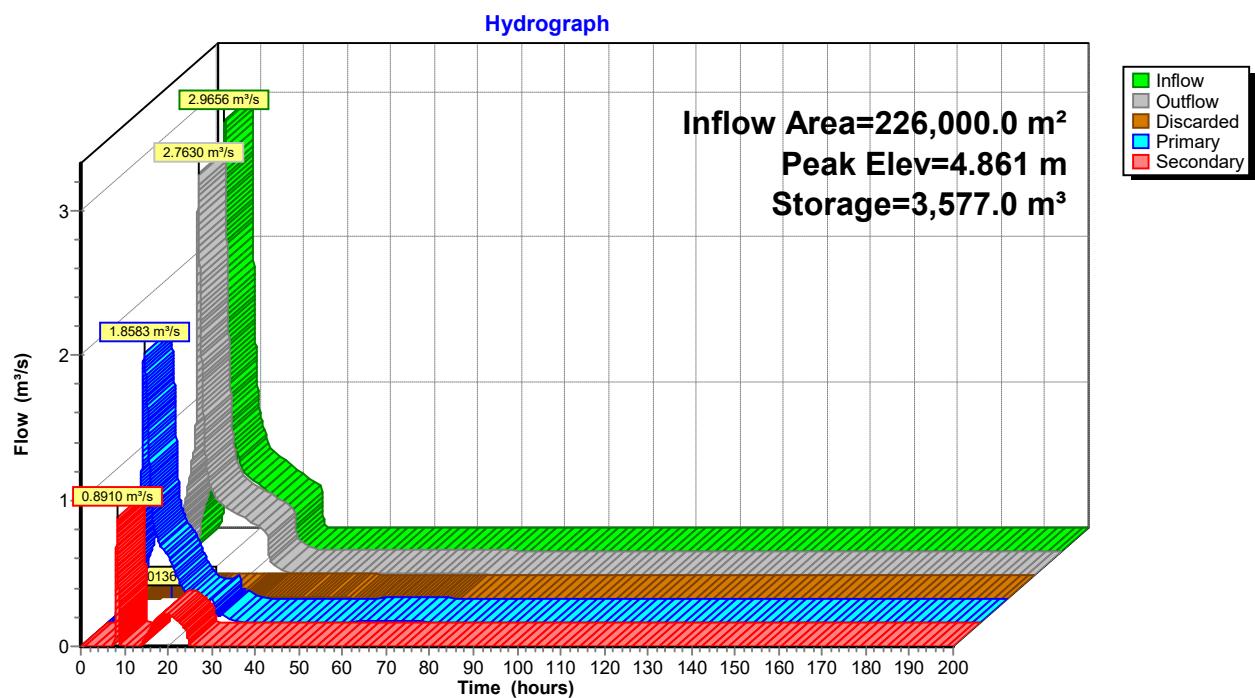
Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	4,636.1 m ³	2.60 mW x 580.00 mL x 1.50 mH 580m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.600 m	1,200 mm Round Culvert L= 15.00 m RCP, sq.cut end projecting, Ke= 0.500 Inlet / Outlet Invert= 3.600 m / 3.600 m S= 0.0000 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 1.131 m ²
#2	Secondary	4.700 m	9.20 m long + 2.0 m/m SideZ x 3.00 m breadth Spillway Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#3	Secondary	5.000 m	580.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#4	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 2,108.8 m ²

Discarded OutFlow Max=0.0136 m³/s @ 8.33 hrs HW=4.861 m (Free Discharge)
 ↗ 4=Exfiltration (Exfiltration Controls 0.0136 m³/s)

Primary OutFlow Max=1.8583 m³/s @ 8.33 hrs HW=4.861 m TW=4.471 m (Dynamic Tailwater)
 ↗ 1=Culvert (Barrel Controls 1.8583 m³/s @ 1.94 m/s)

Secondary OutFlow Max=0.8909 m³/s @ 8.33 hrs HW=4.861 m (Free Discharge)
 ↗ 2=Spillway (Weir Controls 0.8909 m³/s @ 0.58 m/s)
 ↗ 3=Overflow (Controls 0.0000 m³/s)

Pond 130P: Canal - western



12377 - SW Pond - revised (expansion model) R3

Prepared by Hawthorn Geddes Eng & Arch Ltd

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Rainfall Events Listing (selected events)

Event#	Event Name	Storm Type	Curve	Mode	Duration (hours)	B/B	Depth (mm)	AMC
1	5yr Type 1A	Type IA 24-hr		Default	24.00	1	129	2
2	10yr Type 1A	Type IA 24-hr		Default	24.00	1	152	2
3	20yr Type 1A	Type IA 24-hr		Default	24.00	1	176	2
4	100yr Type 1A	Type IA 24-hr		Default	24.00	1	233	2

Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 106 mm for 5yr Type 1A event
 Inflow = 3.9622 m³/s @ 8.25 hrs, Volume= 71,417.5 m³
 Outflow = 3.6806 m³/s @ 8.43 hrs, Volume= 71,389.2 m³, Atten= 7%, Lag= 10.9 min
 Discarded = 0.0214 m³/s @ 24.39 hrs, Volume= 3,174.3 m³
 Primary = 3.6603 m³/s @ 8.43 hrs, Volume= 68,215.0 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.319 m @ 24.39 hrs Surf.Area= 10,310.6 m² Storage= 5,559.3 m³ (4,045.4 m³ above start)

Plug-Flow detention time= 163.2 min calculated for 69,871.8 m³ (98% of inflow)
 Center-of-Mass det. time= 124.1 min (861.1 - 737.0)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

Discarded OutFlow Max=0.0214 m³/s @ 24.39 hrs HW=4.319 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0214 m³/s)

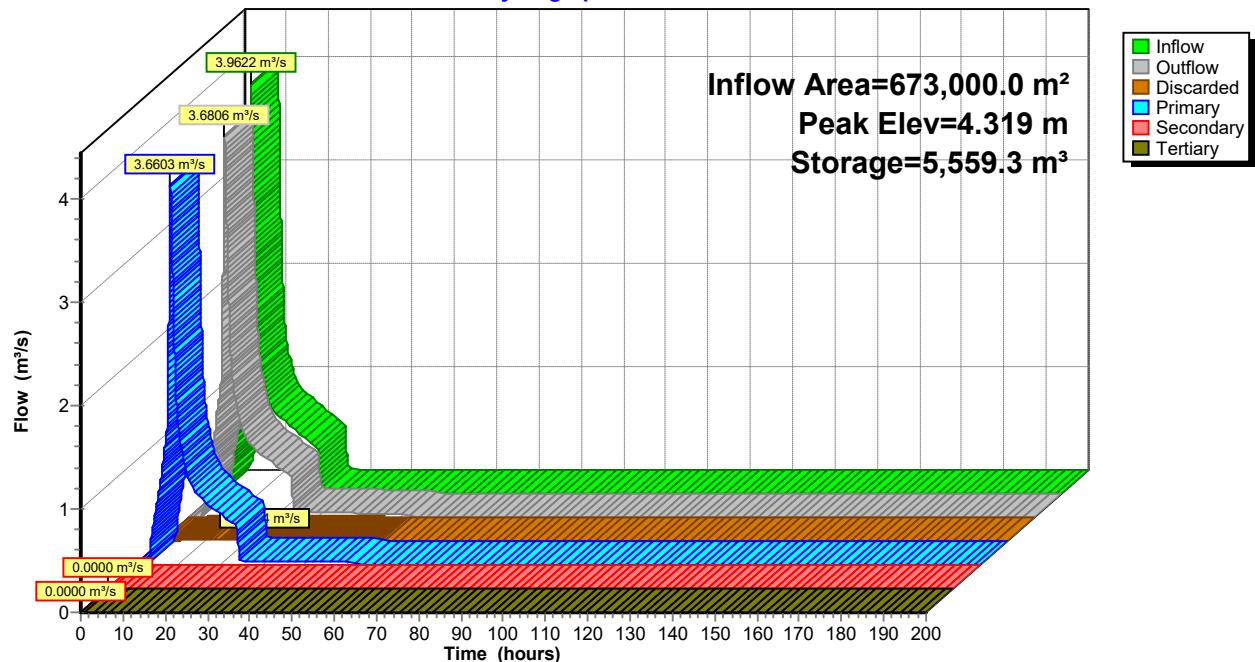
Primary OutFlow Max=3.6601 m³/s @ 8.43 hrs HW=4.297 m TW=3.853 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 3.6601 m³/s @ 1.06 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 4=Proposed Spillway Overflow (Controls 0.0000 m³/s)

Pond 124P: Canal - eastern canal expansion

Hydrograph



Summary for Pond 125P: 825 pipe

Inflow Area = 730,000.0 m², 7.81% Impervious, Inflow Depth = 64 mm for 5yr Type 1A event
 Inflow = 0.1365 m³/s @ 8.49 hrs, Volume= 46,588.3 m³
 Outflow = 0.1364 m³/s @ 8.50 hrs, Volume= 46,585.7 m³, Atten= 0%, Lag= 0.6 min
 Primary = 0.1364 m³/s @ 8.50 hrs, Volume= 46,585.7 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.743 m @ 8.50 hrs Surf.Area= 3.1 m² Storage= 3.6 m³

Plug-Flow detention time= 0.4 min calculated for 46,583.4 m³ (100% of inflow)
 Center-of-Mass det. time= 0.3 min (3,338.8 - 3,338.5)

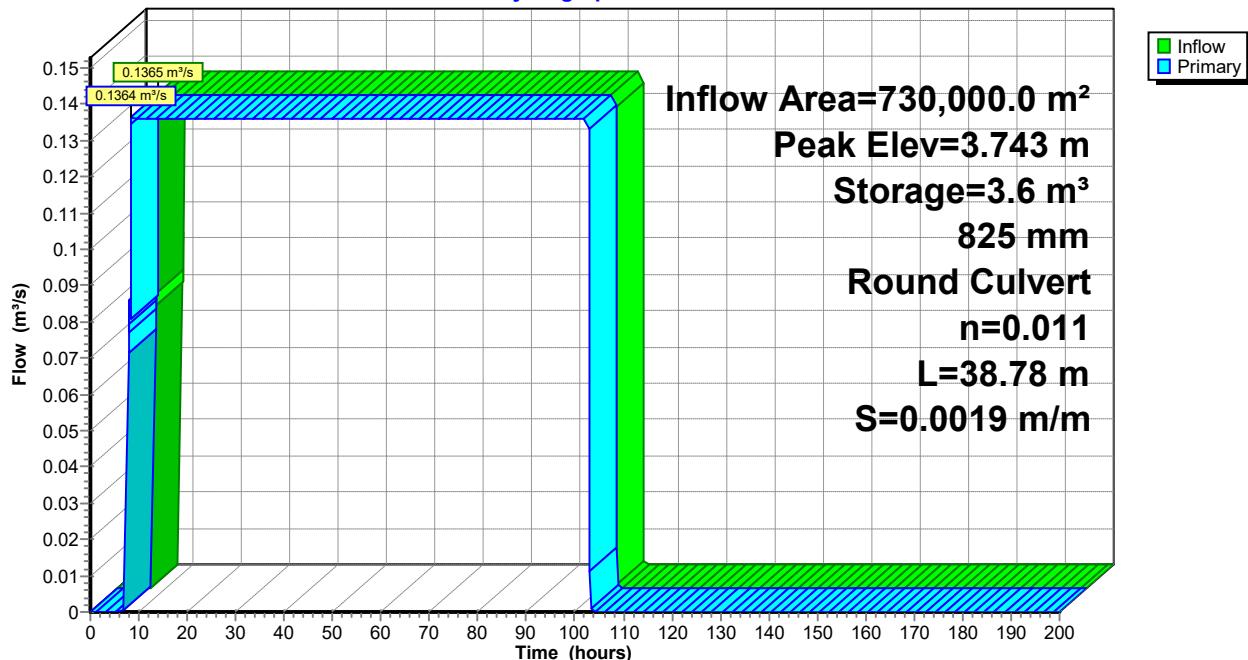
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=0.1364 m³/s @ 8.50 hrs HW=3.743 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.1364 m³/s @ 1.04 m/s)

Pond 125P: 825 pipe

Hydrograph



Summary for Pond 126P: Pond 1

Inflow Area = 695,000.0 m², 3.17% Impervious, Inflow Depth = 102 mm for 5yr Type 1A event
 Inflow = 3.7269 m³/s @ 8.43 hrs, Volume= 70,920.6 m³
 Outflow = 3.6394 m³/s @ 8.55 hrs, Volume= 70,909.9 m³, Atten= 2%, Lag= 7.3 min
 Discarded = 0.0252 m³/s @ 24.41 hrs, Volume= 5,275.5 m³
 Primary = 3.5738 m³/s @ 8.55 hrs, Volume= 46,890.9 m³
 Routed to Pond 123P : Pond 2
 Secondary = 0.0500 m³/s @ 3.28 hrs, Volume= 18,743.5 m³
 Routed to Pond 123P : Pond 2
 Tertiary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Routed to Pond 125P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.300 m Surf.Area= 17,483.6 m² Storage= 3,905.5 m³
 Peak Elev= 4.319 m @ 24.41 hrs Surf.Area= 21,984.1 m² Storage= 24,230.1 m³ (20,324.6 m³ above start)

Plug-Flow detention time= 1,026.9 min calculated for 67,004.5 m³ (94% of inflow)
 Center-of-Mass det. time= 880.0 min (1,659.1 - 779.1)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0252 m³/s @ 24.41 hrs HW=4.319 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0252 m³/s)

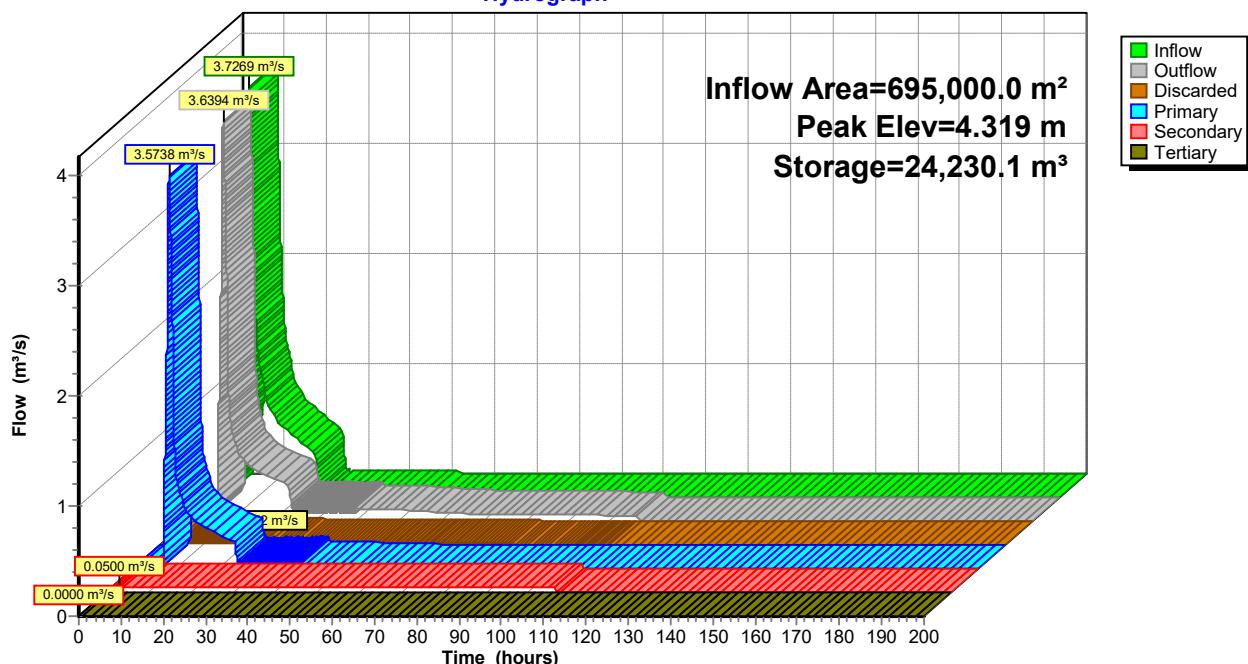
Primary OutFlow Max=3.5737 m³/s @ 8.55 hrs HW=3.855 m TW=3.476 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 3.5737 m³/s @ 0.45 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 3.28 hrs HW=3.300 m TW=3.105 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.300 m TW=2.600 m (Dynamic Tailwater)
 ↗ 4=Culvert (Controls 0.0000 m³/s)
 ↗ 5=Scruffy dome (Controls 0.0000 m³/s)

Pond 126P: Pond 1

Hydrograph



Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 128 mm for 10yr Type 1A event
 Inflow = 4.7361 m³/s @ 8.25 hrs, Volume= 85,948.6 m³
 Outflow = 4.4204 m³/s @ 8.43 hrs, Volume= 85,910.0 m³, Atten= 7%, Lag= 10.5 min
 Discarded = 0.0305 m³/s @ 24.30 hrs, Volume= 4,857.5 m³
 Primary = 4.3980 m³/s @ 8.43 hrs, Volume= 81,018.2 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.0109 m³/s @ 24.30 hrs, Volume= 34.2 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.519 m @ 24.30 hrs Surf.Area= 11,745.6 m² Storage= 7,764.4 m³ (6,250.5 m³ above start)

Plug-Flow detention time= 211.6 min calculated for 84,396.0 m³ (98% of inflow)
 Center-of-Mass det. time= 168.2 min (906.3 - 738.1)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

Discarded OutFlow Max=0.0305 m³/s @ 24.30 hrs HW=4.519 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0305 m³/s)

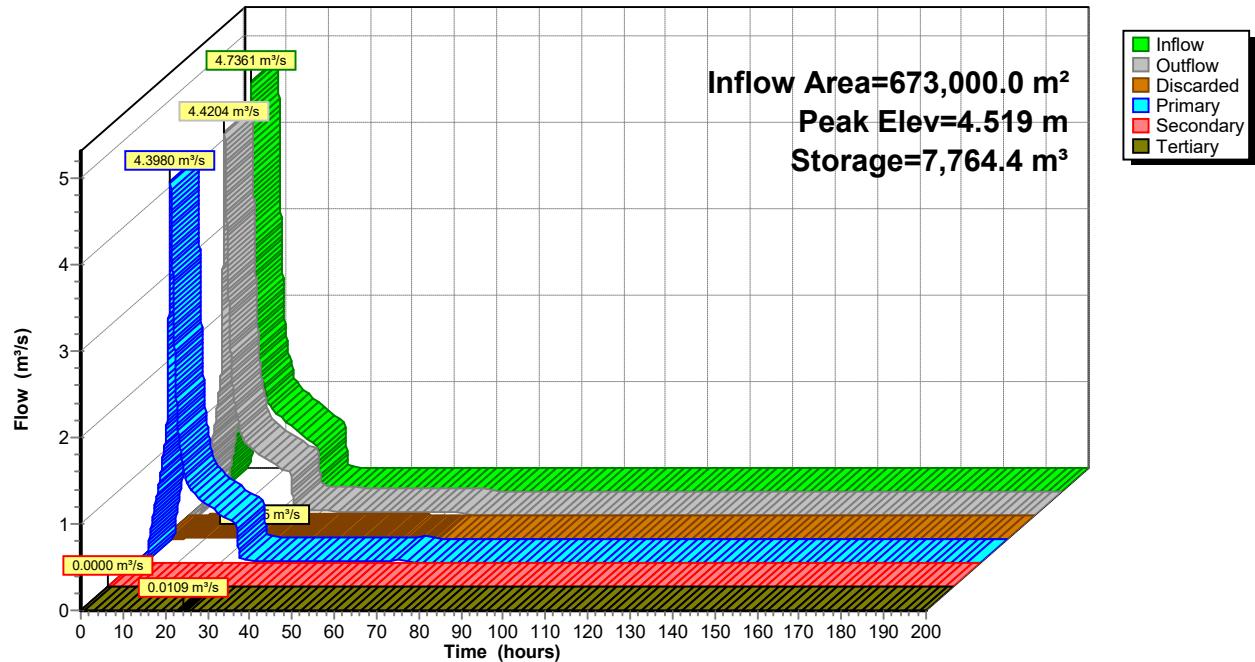
Primary OutFlow Max=4.3979 m³/s @ 8.43 hrs HW=4.342 m TW=3.867 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 4.3979 m³/s @ 1.13 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.0109 m³/s @ 24.30 hrs HW=4.519 m (Free Discharge)
 ↑ 4=Proposed Spillway Overflow (Weir Controls 0.0109 m³/s @ 0.12 m/s)

Pond 124P: Canal - eastern canal expansion

Hydrograph



Summary for Pond 125P: 825 pipe

Inflow Area = 730,000.0 m², 7.81% Impervious, Inflow Depth = 75 mm for 10yr Type 1A event
 Inflow = 0.2637 m³/s @ 24.31 hrs, Volume= 54,958.1 m³
 Outflow = 0.2637 m³/s @ 24.31 hrs, Volume= 54,955.5 m³, Atten= 0%, Lag= 0.0 min
 Primary = 0.2637 m³/s @ 24.31 hrs, Volume= 54,955.5 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 3.880 m @ 24.31 hrs Surf.Area= 3.1 m² Storage= 4.0 m³

Plug-Flow detention time= 0.4 min calculated for 54,952.8 m³ (100% of inflow)
 Center-of-Mass det. time= 0.3 min (3,680.0 - 3,679.7)

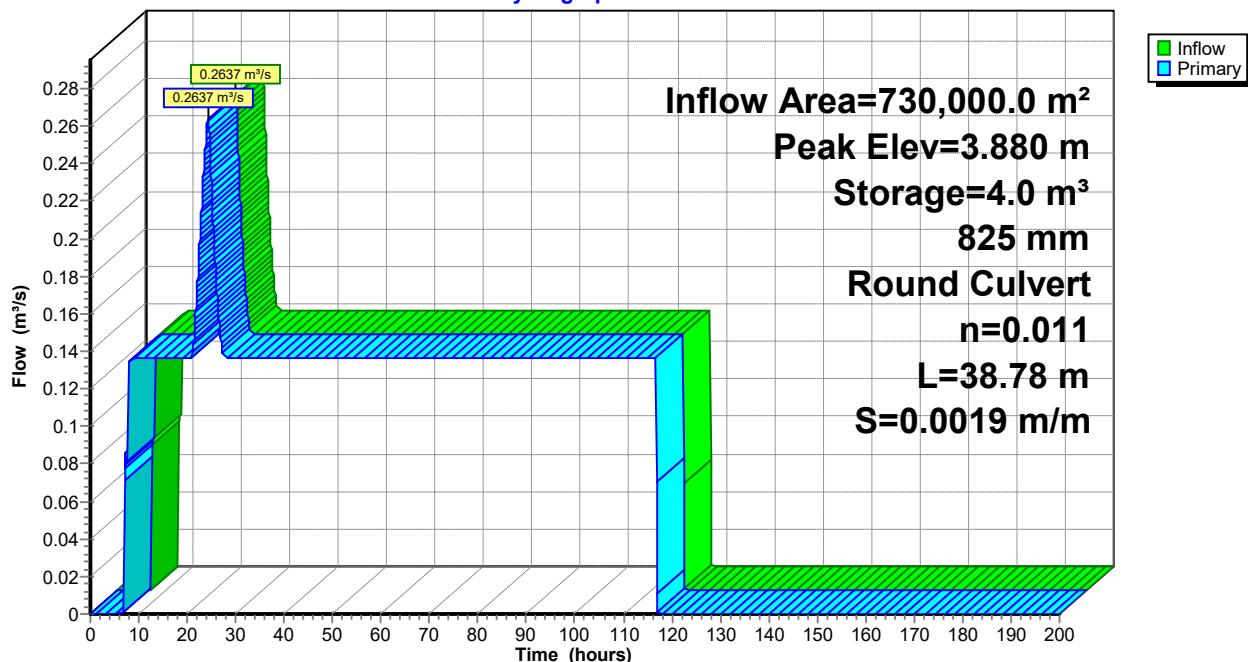
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=0.2637 m³/s @ 24.31 hrs HW=3.880 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.2637 m³/s @ 1.25 m/s)

Pond 125P: 825 pipe

Hydrograph



Summary for Pond 126P: Pond 1

Inflow Area = 695,000.0 m², 3.17% Impervious, Inflow Depth = 121 mm for 10yr Type 1A event
 Inflow = 4.4770 m³/s @ 8.42 hrs, Volume= 84,229.2 m³
 Outflow = 4.3857 m³/s @ 8.54 hrs, Volume= 84,182.7 m³, Atten= 2%, Lag= 6.8 min
 Discarded = 0.0290 m³/s @ 24.31 hrs, Volume= 6,844.9 m³
 Primary = 4.3198 m³/s @ 8.54 hrs, Volume= 54,667.3 m³
 Routed to Pond 123P : Pond 2
 Secondary = 0.0500 m³/s @ 2.91 hrs, Volume= 21,294.2 m³
 Routed to Pond 123P : Pond 2
 Tertiary = 0.1276 m³/s @ 24.31 hrs, Volume= 1,376.4 m³
 Routed to Pond 125P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.300 m Surf.Area= 17,483.6 m² Storage= 3,905.5 m³
 Peak Elev= 4.519 m @ 24.31 hrs Surf.Area= 22,665.1 m² Storage= 28,694.2 m³ (24,788.7 m³ above start)

Plug-Flow detention time= 1,173.7 min calculated for 80,273.2 m³ (95% of inflow)
 Center-of-Mass det. time= 1,008.4 min (1,810.2 - 801.7)

Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic) Listed below

Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0290 m³/s @ 24.31 hrs HW=4.519 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0290 m³/s)

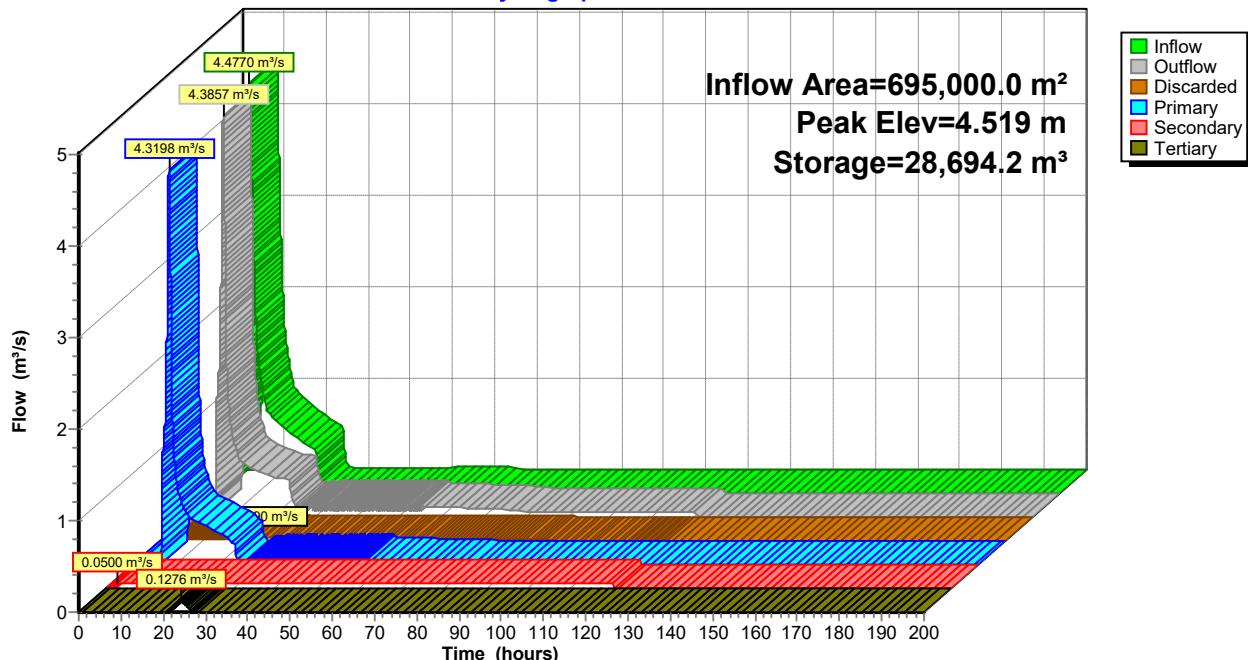
Primary OutFlow Max=4.3196 m³/s @ 8.54 hrs HW=3.869 m TW=3.640 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 4.3196 m³/s @ 0.49 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 2.91 hrs HW=3.300 m TW=3.107 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.1276 m³/s @ 24.31 hrs HW=4.519 m TW=3.880 m (Dynamic Tailwater)
 ↗ 4=Culvert (Passes 0.1276 m³/s of 0.9782 m³/s potential flow)
 ↗ 5=Scruffy dome (Weir Controls 0.1276 m³/s @ 0.40 m/s)

Pond 126P: Pond 1

Hydrograph



Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 151 mm for 20yr Type 1A event
 Inflow = 5.5381 m³/s @ 8.25 hrs, Volume= 101,376.4 m³
 Outflow = 5.1743 m³/s @ 8.42 hrs, Volume= 101,336.2 m³, Atten= 7%, Lag= 10.1 min
 Discarded = 0.0328 m³/s @ 20.65 hrs, Volume= 5,368.4 m³
 Primary = 5.1499 m³/s @ 8.42 hrs, Volume= 91,245.9 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.1959 m³/s @ 20.65 hrs, Volume= 4,722.0 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.569 m @ 20.65 hrs Surf.Area= 12,109.6 m² Storage= 8,369.1 m³ (6,855.2 m³ above start)

Plug-Flow detention time= 199.3 min calculated for 99,822.3 m³ (98% of inflow)
 Center-of-Mass det. time= 161.3 min (894.9 - 733.6)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

Discarded OutFlow Max=0.0328 m³/s @ 20.65 hrs HW=4.569 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0328 m³/s)

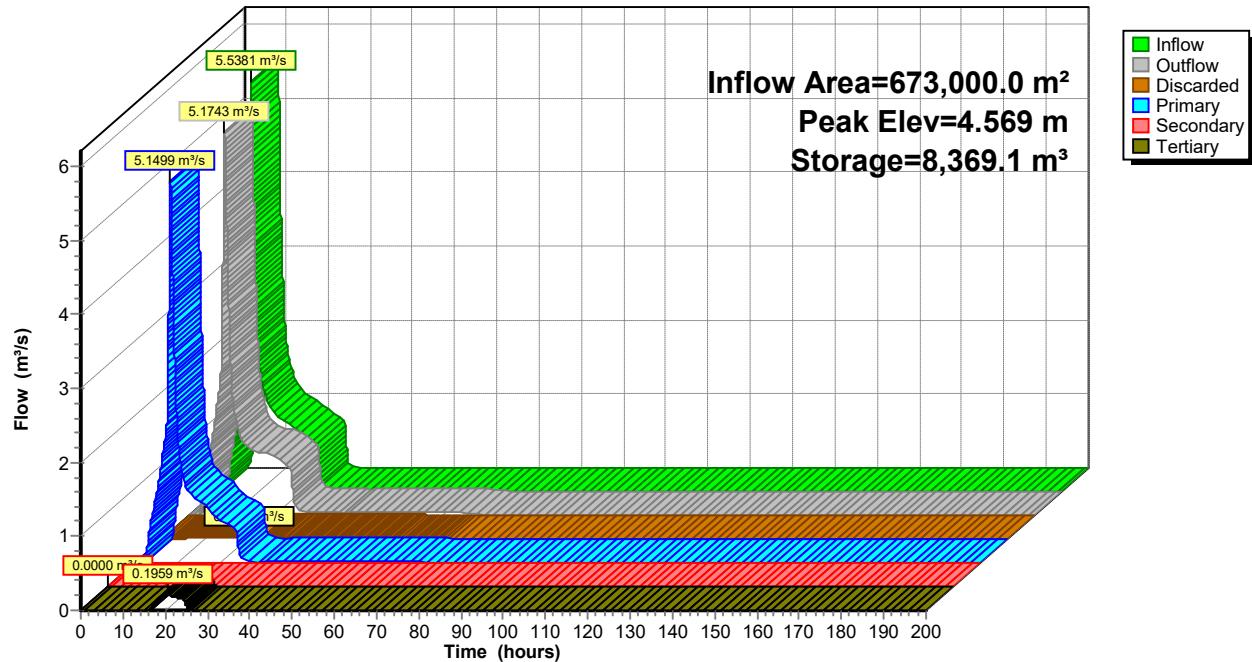
Primary OutFlow Max=5.1497 m³/s @ 8.42 hrs HW=4.386 m TW=3.880 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 5.1497 m³/s @ 1.19 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.1959 m³/s @ 20.65 hrs HW=4.569 m (Free Discharge)
 ↑ 4=Proposed Spillway Overflow (Weir Controls 0.1959 m³/s @ 0.33 m/s)

Pond 124P: Canal - eastern canal expansion

Hydrograph



Summary for Pond 125P: 825 pipe

Inflow Area = 730,000.0 m², 7.81% Impervious, Inflow Depth = 90 mm for 20yr Type 1A event
 Inflow = 0.5079 m³/s @ 20.67 hrs, Volume= 65,505.3 m³
 Outflow = 0.5079 m³/s @ 20.67 hrs, Volume= 65,502.7 m³, Atten= 0%, Lag= 0.1 min
 Primary = 0.5079 m³/s @ 20.67 hrs, Volume= 65,502.7 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.091 m @ 20.67 hrs Surf.Area= 3.1 m² Storage= 4.7 m³

Plug-Flow detention time= 0.4 min calculated for 65,499.5 m³ (100% of inflow)
 Center-of-Mass det. time= 0.2 min (3,357.0 - 3,356.7)

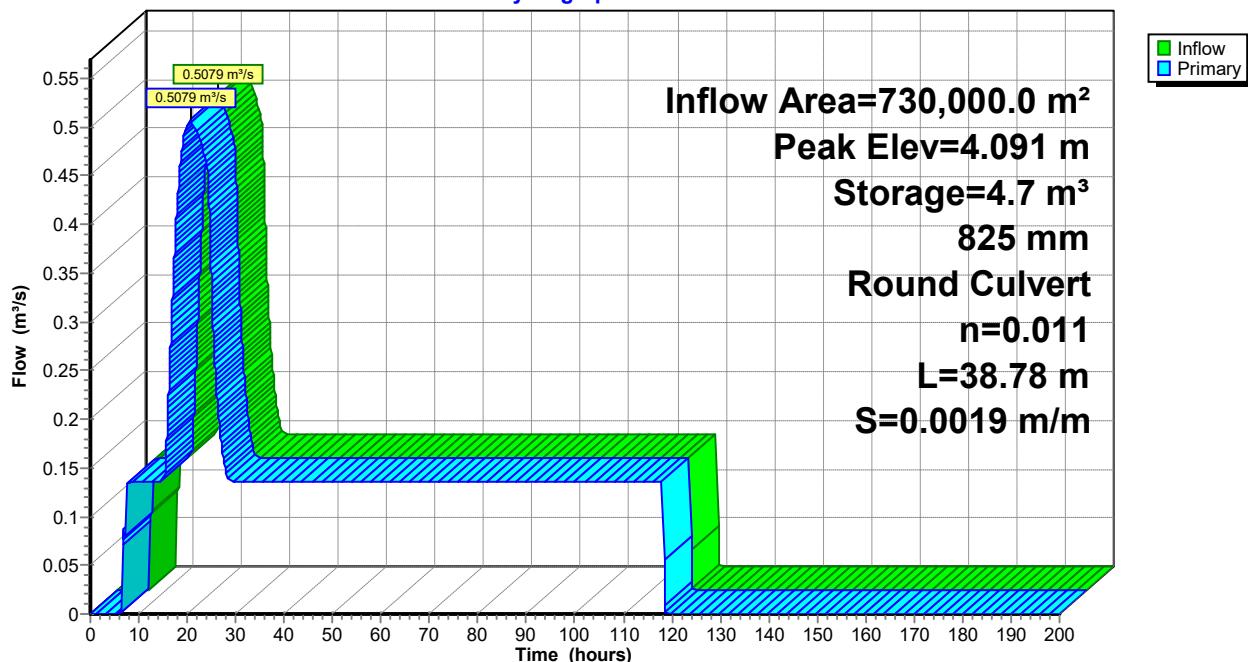
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=0.5079 m³/s @ 20.67 hrs HW=4.091 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.5079 m³/s @ 1.49 m/s)

Pond 125P: 825 pipe

Hydrograph



Summary for Pond 126P: Pond 1

Inflow Area = 695,000.0 m², 3.17% Impervious, Inflow Depth = 137 mm for 20yr Type 1A event
 Inflow = 5.2425 m³/s @ 8.41 hrs, Volume= 94,984.3 m³
 Outflow = 5.0746 m³/s @ 8.44 hrs, Volume= 94,940.6 m³, Atten= 3%, Lag= 1.4 min
 Discarded = 0.0300 m³/s @ 20.67 hrs, Volume= 7,188.9 m³
 Primary = 5.0085 m³/s @ 8.44 hrs, Volume= 55,239.5 m³
 Routed to Pond 123P : Pond 2
 Secondary = 0.0500 m³/s @ 2.62 hrs, Volume= 21,646.7 m³
 Routed to Pond 123P : Pond 2
 Tertiary = 0.3717 m³/s @ 20.67 hrs, Volume= 10,865.5 m³
 Routed to Pond 125P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.300 m Surf.Area= 17,483.6 m² Storage= 3,905.5 m³
 Peak Elev= 4.569 m @ 20.67 hrs Surf.Area= 22,837.2 m² Storage= 29,836.0 m³ (25,930.5 m³ above start)

Plug-Flow detention time= 1,100.8 min calculated for 91,035.2 m³ (96% of inflow)
 Center-of-Mass det. time= 952.1 min (1,729.7 - 777.7)

Volume	Invert	Avail.Storage	Storage Description	
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic)	Listed below
Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0300 m³/s @ 20.67 hrs HW=4.569 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0300 m³/s)

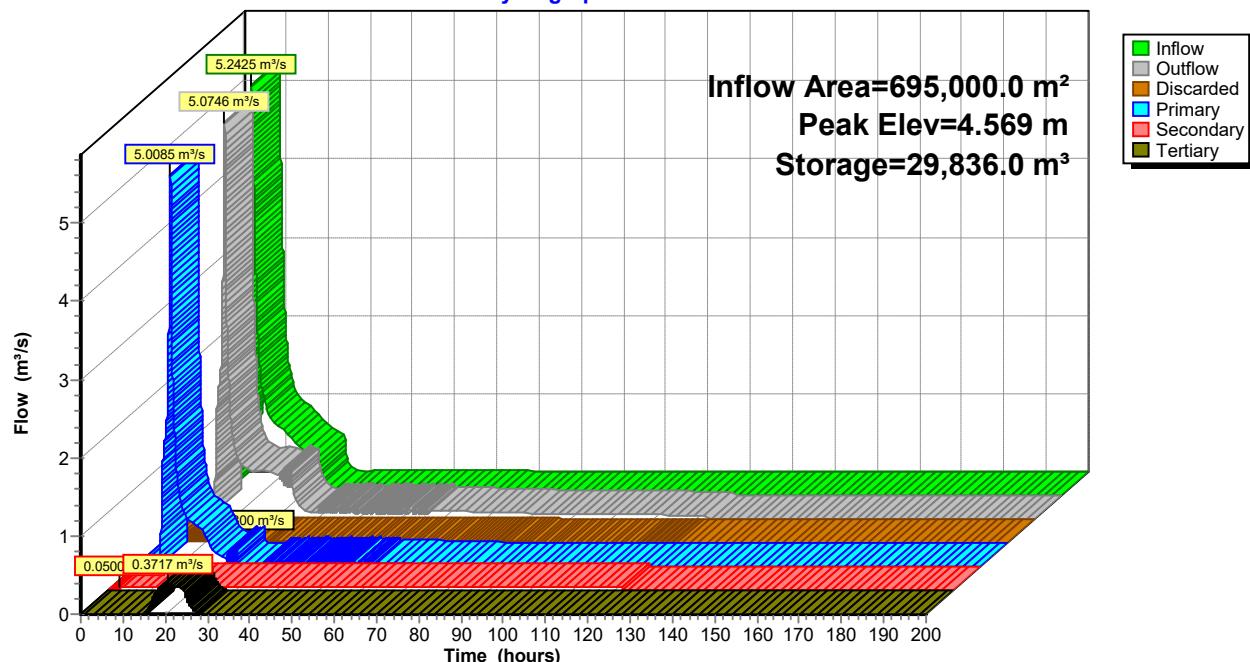
Primary OutFlow Max=4.9814 m³/s @ 8.44 hrs HW=3.880 m TW=3.759 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 4.9814 m³/s @ 0.51 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 2.62 hrs HW=3.300 m TW=3.108 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.3717 m³/s @ 20.67 hrs HW=4.569 m TW=4.091 m (Dynamic Tailwater)
 ↗ 4=Culvert (Passes 0.3717 m³/s of 0.9819 m³/s potential flow)
 ↗ 5=Scruffy dome (Weir Controls 0.3717 m³/s @ 0.57 m/s)

Pond 126P: Pond 1

Hydrograph



Summary for Pond 124P: Canal - eastern canal expansion

Inflow Area = 673,000.0 m², 0.00% Impervious, Inflow Depth = 204 mm for 100yr Type 1A event
 Inflow = 7.2637 m³/s @ 8.23 hrs, Volume= 137,419.8 m³
 Outflow = 6.6652 m³/s @ 8.36 hrs, Volume= 137,378.9 m³, Atten= 8%, Lag= 7.7 min
 Discarded = 0.0353 m³/s @ 14.50 hrs, Volume= 5,846.3 m³
 Primary = 6.6367 m³/s @ 8.36 hrs, Volume= 110,114.0 m³
 Routed to Pond 126P : Pond 1
 Secondary = 0.0000 m³/s @ 0.00 hrs, Volume= 0.0 m³
 Tertiary = 0.5634 m³/s @ 14.50 hrs, Volume= 21,418.7 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.850 m Surf.Area= 6,950.7 m² Storage= 1,513.9 m³
 Peak Elev= 4.625 m @ 14.50 hrs Surf.Area= 12,511.6 m² Storage= 9,058.3 m³ (7,544.3 m³ above start)

Plug-Flow detention time= 160.9 min calculated for 135,865.0 m³ (99% of inflow)
 Center-of-Mass det. time= 132.6 min (855.6 - 723.0)

Volume	Invert	Avail.Storage	Storage Description
#1	3.600 m	15,805.9 m ³	2.60 mW x 1,985.00 mL x 1.50 mH 1985m canal Z=1.8
Device	Routing	Invert	Outlet Devices
#1	Primary	3.950 m	10.00 m long Weir 2 End Contraction(s)
#2	Secondary	5.000 m	620.00 m long x 0.30 m breadth Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 Coef. (Metric) 1.49 1.50 1.52 1.58 1.65 1.70 1.77 1.81 1.83 1.82 1.83
#3	Discarded	3.850 m	20.00 mm/hr Exfiltration over Wetted area above 3.850 m Excluded Wetted area = 7,208.3 m ²
#4	Tertiary	4.510 m	10.00 m long + 2.0 m/m SideZ x 1.00 m breadth Proposed Spillway Overflow Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 0.549 0.610 0.762 0.914 1.067 1.219 1.372 1.524 1.676 Coef. (Metric) 1.34 1.42 1.48 1.48 1.47 1.46 1.46 1.48 1.48 1.50 1.54 1.58 1.61 1.65 1.76 1.79 1.83

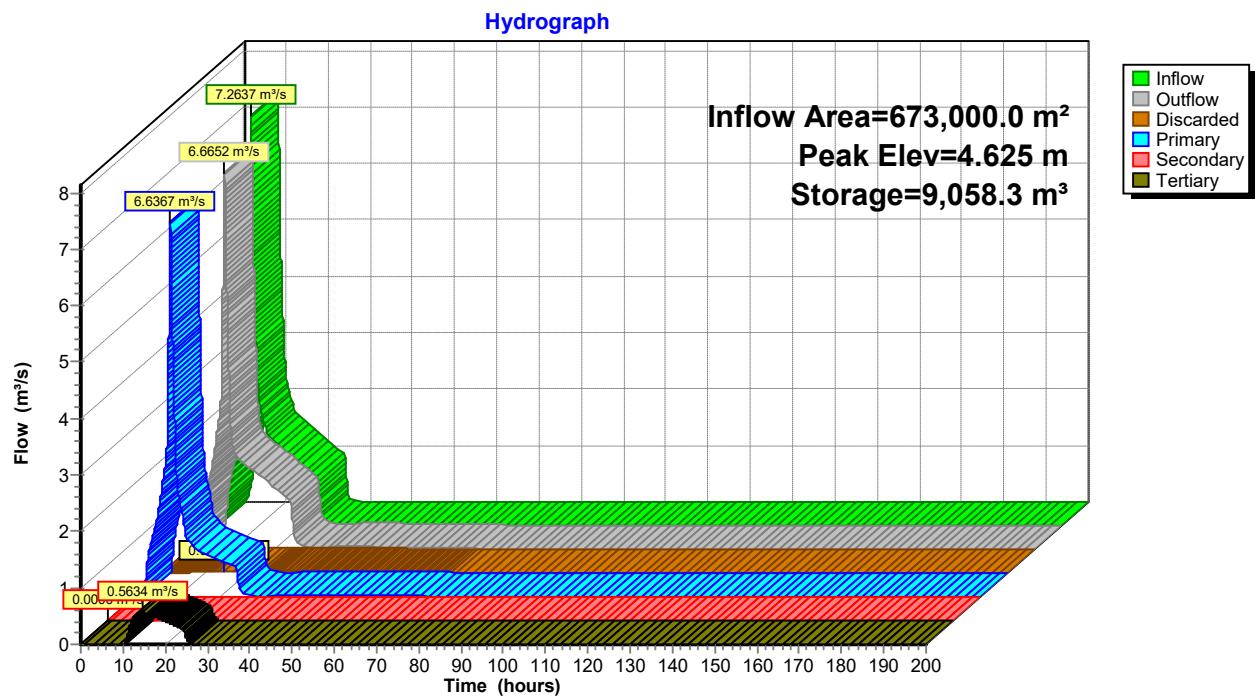
Discarded OutFlow Max=0.0353 m³/s @ 14.50 hrs HW=4.625 m (Free Discharge)
 ↑ 3=Exfiltration (Exfiltration Controls 0.0353 m³/s)

Primary OutFlow Max=6.6241 m³/s @ 8.36 hrs HW=4.473 m TW=4.026 m (Dynamic Tailwater)
 ↑ 1=Weir (Weir Controls 6.6241 m³/s @ 1.28 m/s)

Secondary OutFlow Max=0.0000 m³/s @ 0.00 hrs HW=3.850 m (Free Discharge)
 ↑ 2=Overflow (Controls 0.0000 m³/s)

Tertiary OutFlow Max=0.5634 m³/s @ 14.50 hrs HW=4.625 m (Free Discharge)
 ↑ 4=Proposed Spillway Overflow (Weir Controls 0.5634 m³/s @ 0.48 m/s)

Pond 124P: Canal - eastern canal expansion



Summary for Pond 125P: 825 pipe

Inflow Area = 730,000.0 m², 7.81% Impervious, Inflow Depth = 119 mm for 100yr Type 1A event
 Inflow = 0.8683 m³/s @ 14.53 hrs, Volume= 86,731.2 m³
 Outflow = 0.8691 m³/s @ 14.63 hrs, Volume= 86,728.6 m³, Atten= 0%, Lag= 5.9 min
 Primary = 0.8691 m³/s @ 14.63 hrs, Volume= 86,728.6 m³

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Peak Elev= 4.399 m @ 14.63 hrs Surf.Area= 3.1 m² Storage= 5.7 m³

Plug-Flow detention time= 0.3 min calculated for 86,724.2 m³ (100% of inflow)
 Center-of-Mass det. time= 0.2 min (2,799.0 - 2,798.8)

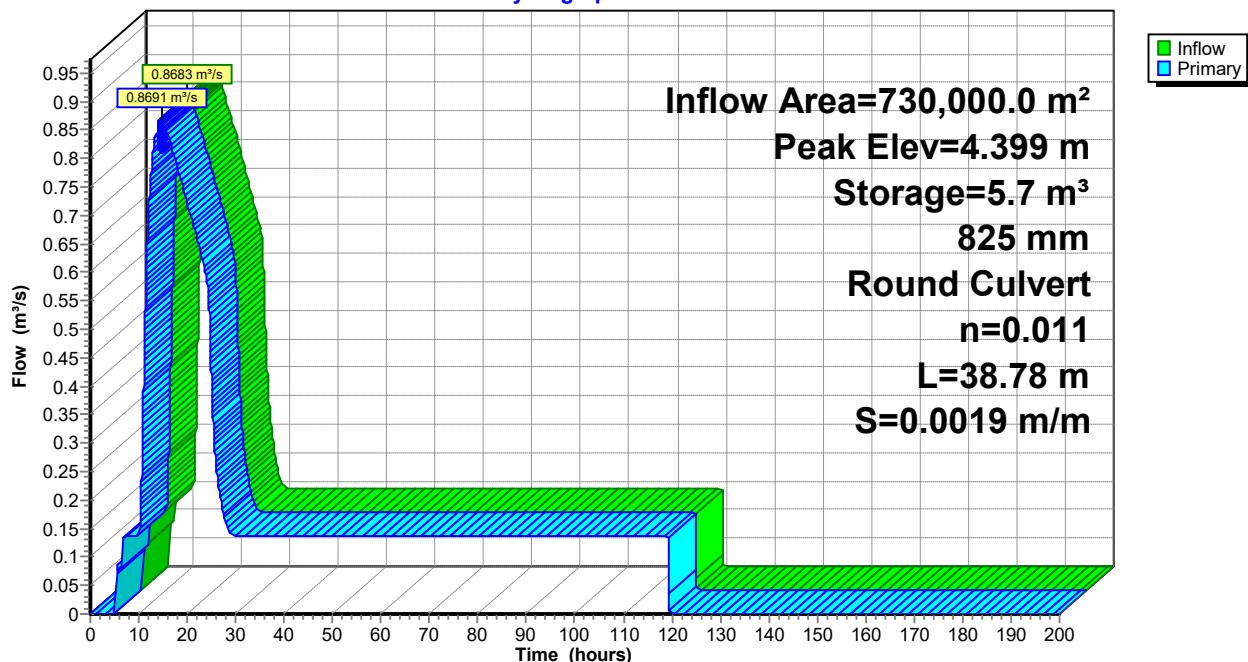
Volume	Invert	Avail.Storage	Storage Description
#1	2.600 m	15.7 m ³	2.00 mD x 5.00 mH Vertical Cone/Cylinder

Device	Routing	Invert	Outlet Devices
#1	Primary	3.420 m	825 mm Round Culvert L= 38.78 m Ke= 0.500 Inlet / Outlet Invert= 3.420 m / 3.346 m S= 0.0019 m/m Cc= 0.900 n= 0.011 Concrete pipe, straight & clean, Flow Area= 0.535 m ²

Primary OutFlow Max=0.8645 m³/s @ 14.63 hrs HW=4.395 m (Free Discharge)
 ↑—1=Culvert (Barrel Controls 0.8645 m³/s @ 1.73 m/s)

Pond 125P: 825 pipe

Hydrograph



Summary for Pond 126P: Pond 1

Inflow Area = 695,000.0 m², 3.17% Impervious, Inflow Depth = 166 mm for 100yr Type 1A event
 Inflow = 6.7678 m³/s @ 8.35 hrs, Volume= 115,105.6 m³
 Outflow = 4.2153 m³/s @ 8.30 hrs, Volume= 115,041.3 m³, Atten= 38%, Lag= 0.0 min
 Discarded = 0.0310 m³/s @ 14.53 hrs, Volume= 7,486.5 m³
 Primary = 4.1466 m³/s @ 8.30 hrs, Volume= 54,500.8 m³
 Routed to Pond 123P : Pond 2
 Secondary = 0.0500 m³/s @ 2.16 hrs, Volume= 21,898.5 m³
 Routed to Pond 123P : Pond 2
 Tertiary = 0.7249 m³/s @ 14.53 hrs, Volume= 31,155.5 m³
 Routed to Pond 125P : 825 pipe

Routing by Dyn-Stor-Ind method, Time Span= 0.00-200.00 hrs, dt= 0.01 hrs
 Starting Elev= 3.300 m Surf.Area= 17,483.6 m² Storage= 3,905.5 m³
 Peak Elev= 4.625 m @ 14.53 hrs Surf.Area= 23,026.9 m² Storage= 31,108.8 m³ (27,203.3 m³ above start)

Plug-Flow detention time= 948.9 min calculated for 111,130.3 m³ (97% of inflow)
 Center-of-Mass det. time= 828.1 min (1,562.1 - 734.0)

Volume	Invert	Avail.Storage	Storage Description	
#1	2.600 m	50,024.2 m ³	Custom Stage Data (Conic)	Listed below
Elevation (meters)	Surf.Area (sq-meters)	Inc.Store (cubic-meters)	Cum.Store (cubic-meters)	Wet.Area (sq-meters)
2.600	31.5	0.0	0.0	31.5
2.800	605.0	51.6	51.6	605.1
3.000	2,950.9	326.1	377.8	2,951.2
3.200	16,722.6	1,779.9	2,157.7	16,723.0
3.400	18,244.7	3,495.6	5,653.3	18,248.0
3.600	19,084.6	3,732.6	9,385.9	19,093.5
3.800	20,068.2	3,914.9	13,300.8	20,082.1
4.000	20,826.6	4,089.2	17,390.0	20,847.2
4.200	21,580.7	4,240.5	21,630.5	21,608.3
4.400	22,261.0	4,384.0	26,014.5	22,296.7
4.600	22,942.6	4,520.2	30,534.7	22,986.6
4.800	23,626.5	4,656.7	35,191.4	23,679.0
5.000	24,332.5	4,795.7	39,987.2	24,393.5
5.200	25,108.0	4,943.8	44,931.0	25,176.9
5.400	25,826.0	5,093.2	50,024.2	25,903.8

Device	Routing	Invert	Outlet Devices
#1	Primary	3.750 m	75.00 m long x 3.00 m breadth Weir Head (meters) 0.061 0.122 0.183 0.244 0.305 0.366 0.427 0.488 Coef. (Metric) 1.37 1.41 1.49 1.48 1.48 1.48 1.47 1.46
#2	Discarded	3.300 m	20.00 mm/hr Exfiltration over Wetted area above 3.300 m Excluded Wetted area = 17,485.5 m ²
#3	Secondary	3.300 m	0.0500 m³/s Through bund when above 3.300 m
#4	Tertiary	3.449 m	825 mm Round Culvert L= 15.00 m Ke= 0.500 Inlet / Outlet Invert= 3.449 m / 3.420 m S= 0.0019 m/m Cc= 0.900 n= 0.010 PVC, smooth interior, Flow Area= 0.535 m ²
#5	Device 4	4.470 m	2,100 mm Horiz. Scruffy dome C= 0.600 Limited to weir flow at low heads

Discarded OutFlow Max=0.0310 m³/s @ 14.53 hrs HW=4.625 m (Free Discharge)
 ↗ 2=Exfiltration (Exfiltration Controls 0.0310 m³/s)

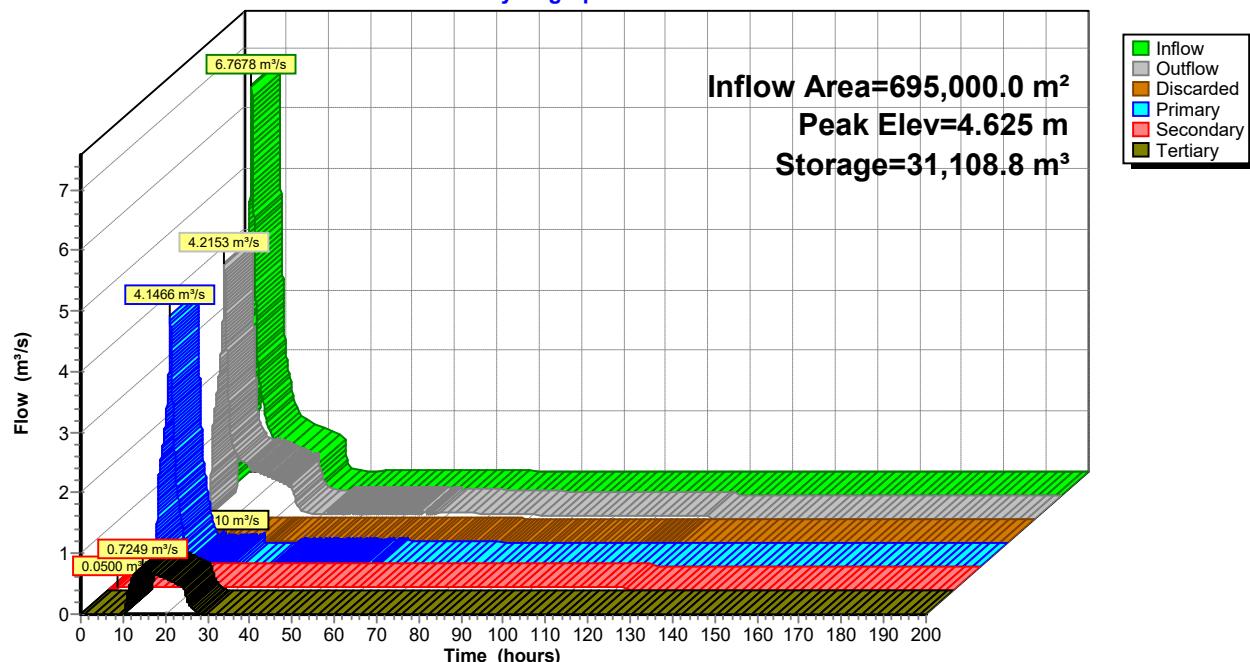
Primary OutFlow Max=2.7906 m³/s @ 8.30 hrs HW=3.999 m TW=3.997 m (Dynamic Tailwater)
 ↗ 1=Weir (Weir Controls 2.7906 m³/s @ 0.15 m/s)

Secondary OutFlow Max=0.0500 m³/s @ 2.16 hrs HW=3.300 m TW=3.111 m (Dynamic Tailwater)
 ↗ 3=Through bund (Exfiltration Controls 0.0500 m³/s)

Tertiary OutFlow Max=0.6935 m³/s @ 14.53 hrs HW=4.625 m TW=4.386 m (Dynamic Tailwater)
 ↗ 4=Culvert (Inlet Controls 0.6935 m³/s @ 1.30 m/s)
 ↗ 5=Scruffy dome (Passes 0.6935 m³/s of 0.7249 m³/s potential flow)

Pond 126P: Pond 1

Hydrograph



Appendix B – Consent CON20090505532 – Schedule 1

SCHEDULE 1

MONITORING PROGRAMME – RESOURCE CONSENT CON20090505532

The Consent Holder shall undertake the monitoring as follows:

1 WATER QUALITY OF DISCHARGES FROM THE STORMWATER SETTLEMENT AND STORAGE POND SYSTEM

1.1 Routine Water Monitoring for Discharges from the stormwater settlement and storage pond to Whangarei Harbour

The stormwater system and discharges shall be monitored in accordance with Table 1 attached below

If any of the following determinants in the stormwater being discharged to the coastal marine area exceed the Action Values specified in Table A, the Consent Holder will notify the NRC within two weeks of receiving the sample result and investigate the source of the contaminant and advise the NRC as to the findings of the investigation and any management response.

Table A

Determinands	Action values: Concentration in milligrams per cubic metre
Total Aluminium	5
Total copper	13
Total lead	44
Total zinc	150
PAHs	
– Acenaphthene	58
– Anthracene	0.1
– Benzo(α)anthracene	0.18
– Benzo(α)pyrene	0.1
– Fluoranthene	10
– Fluorene	30
– Naphthalene	500
– Phenanthrene	6
– Pyrene	0.25

Note: ANZECC for PAH, 99% protection level as recommended in Section 8.3.7.7 and also CEQG (Canadian aquatic guidelines). For aluminium, ANZECC 8.3.7 Marine guidelines recommend 0.5 mg/m as an indicative low reliability figure.

Values in Table A are intended to act as an early warning to identify if concentrations are increasing relative to previously documented monitoring values/trends and warrant investigation notwithstanding that they may be well below levels of environmental concern taking into account mixing and dilution.

TABLE 1: SCHEMATIC MONITORING DIAGRAM –

Location	Sampling Frequency	Parameters	Criteria	Notes
Point of discharge from treatment pond system	First discharge per season, and two other discharge events each year	TSS, VSS, NTU and pH	TSS as in Condition 5(b)	Advise NRC when ponds reach design discharge level for the first time each year prior to discharge occurring
Three samples spaced evenly over each day (operational hours) until discharge has ceased. First sample to be taken as close as possible to when discharge first occurs.			T and DO are considered not useful in this situation as they will reflect conditions intrinsic to the wetland and in any event cannot have any influence on water quality in this particular marine receiving environment.	
Taken with first sample from first discharge event only.	Al, Cu, Pb, Zn, PAH, and resin acids. Total N and Total P to be included if fertiliser products have been stored on site in the previous season.		<u>Action values</u> see table A in 1.1 above. Resin acids, Total N and P concentrations assessed against literature and concentrations to determine potential for adverse effects. All parameters to be assessed for any increasing trends over time.	If the resin acid results for the first discharge of the season are below any applicable ANZECC effect threshold after theoretical mixing, resin acids need not be further analysed in that season.
One-off under existing regime	WETT (Toxicity Testing)		As specified in point 1.3 below	One further WETT will be undertaken under the present port conditions. The need for any further WETT will be considered only if new port operations introduce new contaminant(s) into the stormwater.
Pond Influent	To be done with "First discharge per season" referred to above	T, pH, DO, TSS, Cu, Pb, Zn, resin acids, phenols, PAH, VSS	Trend data only, no compliance limits.	Test to be used as an indication of pond effectiveness under different conditions eg size of storm, contributing area

Stormwater Canals, western/eastern arms	One off	Sediment samples: Cu, Pb, Zn, PAH	Trend data only but reference to ANZECC ISQG values to assess pollution status. <u>Water</u> · Winter months (when ponding in canals following rainfall) pH, Cu, Pb, Zn, resin acids, phenols, PAH	Samples to be taken at: Join of arms, 100m upstream on eastern arm, 100m upstream on western arm Test to be used to determine any disposal issues for sediment
Groundwater				Both sediment and water samples to be representative based on 3 subsamples from different points of each arm composited for analytical purposes

Abbreviations

ANZECC	The Australian and New Zealand Environment and Conservation Council
T	Temperature
DO	Dissolved oxygen (both g/m ³ and % saturation)
TSS	Total Suspended Solids
Total N	Total Nitrogen
Total P	Total Phosphorus
FC	Faecal Coliforms
Cu	Copper
Pb	Lead
Zn	Zinc
PAH	Polycyclic aromatic hydrocarbon
WETT	Whole Effluent Toxicity Test
VSS	Volatile Suspended Solids
NTU	Nephelometric Turbidity Unit

1.2 Pumping Hours

The Consent Holder shall measure the pumping hours, the date, the time, and the quantity of water when the discharge to Whangarei Harbour occurs.

Advice Note: *The application states that the approximately average volume of stormwater to be discharged is assessed at 200,000 cubic metres per annum. The size of the discharge pipe and the proposed capacity of the pumps limit the pumped discharge rate to approximately 2,520 cubic metres per hour.*

1.3 Wett Method

The WETT method for toxicity analyses shall be undertaken on not less than three representative marine species, including at least one algae, one invertebrate, and one fish. The choice of toxicity test species, dilutions, test endpoints to be measured, and "toxicity effect" shall be submitted to the Council for approval at least twenty working days prior to stormwater sampling. For each of the three [3] toxicity tests the EC₂₅ (the concentration of stormwater estimated to produce a toxic effect in 25% of the test organisms) shall be greater than the equivalent of a 200-fold dilution of the stormwater. The dilution water used for toxicity tests shall be an uncontaminated sample of Whangarei Harbour water, collected on an incoming tide at the harbour entrance, at a point agreed to by the Council. There shall be no significant toxicity after a 200-fold dilution of the stormwater. For the purposes of this condition "significant toxicity" is defined as no more than a 25% toxic effect measured in the most sensitive test species used. Testing of the samples shall be carried out in accordance with the methodology outlined in the NIWA document entitled "Standard Methods for Whole Effluent Toxicity Testing: Development and Application" dated November 1998.

- 1.4** The pH and TSS results taken in accordance with Table 1 will be recorded in an ongoing spreadsheet a copy of which shall be forwarded to the Council Monitoring Manager as required by Condition 2 below. Any results recorded which do not achieve the criteria included in Condition 5 shall be reported to the Council Monitoring Manager together with an explanation within seven days of their receipt by Northport.

2 REPORTING

- 2.1** The Consent Holder shall forward to the Council Monitoring Manager by 31 August each year an annual report for the previous period 1 July to 30 June detailing the results of the monitoring required by Section 1 of this monitoring programme and an assessment of compliance with the conditions of consent.

3 REVIEW

The Regional Council, in conjunction with the Consent Holder, may undertake a review of the monitoring programme every two years. The review will take into account the Consent Holders monitoring results, any monitoring undertaken by the Regional Council and the level of development within the catchment areas. The Consent Holder shall meet the reasonable costs of any such review.

4 FIELD MEASUREMENTS, RECORDS, SAMPLE COLLECTION, SAMPLE TRANSPORT, DETECTION LIMITS, AND LABORATORY REQUIREMENTS

4.1 Records

A record of rainfall conditions preceding and during sampling shall be kept. This record shall be based on a nearby rainfall recording site agreed by the Council.

4.2 Sample Collection

All samples collected as part of this monitoring programme shall be collected using standard methods and approved containers.

4.3 Sample Transport

All samples collected as part of this monitoring programme shall be transported in accordance with standard procedures and under chain of custody to the laboratory.

4.4 Detection Limits

The detection limits for the analysis of metals in sediment and water samples collected shall be equivalent to, or better than, those specified below:

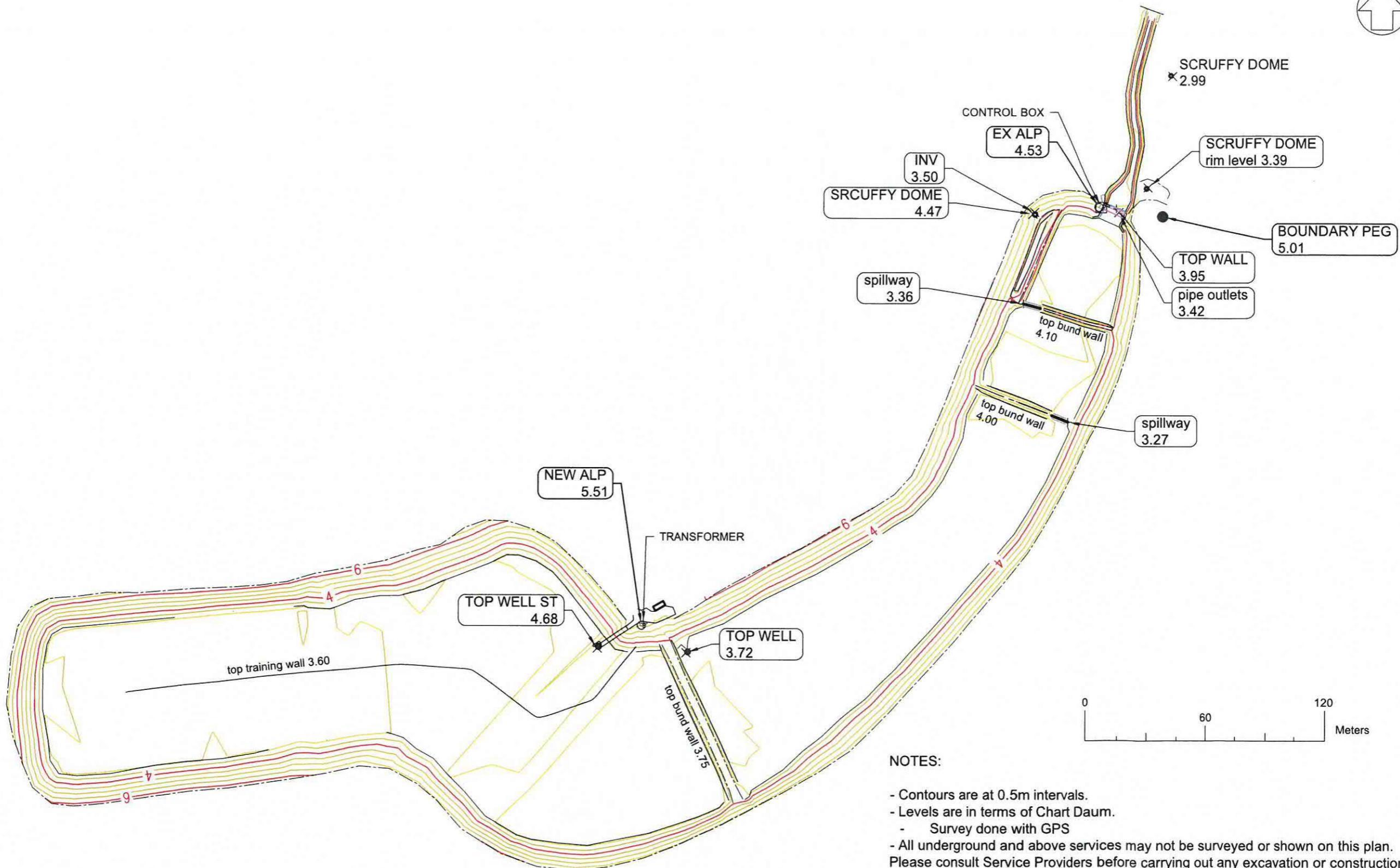
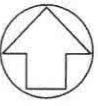
Metal	Sediment samples (milligrams per kilogram)	Water samples (milligrams per cubic metre)
total copper	2	1.0
total lead	0.4	0.2
total zinc	4	2.0
total arsenic	2	N/A
total cadmium	0.1	N/A
total chromium	2	N/A

4.5 Laboratory Requirements

All samples collected as part of this monitoring programme shall be analysed at a laboratory with registered quality assurance procedures (see definition below), and all analyses shall be conducted using standard methods.

Registered quality assurance procedures are procedures that ensure that the laboratory meets good management practices and would include registrations such as ISO 9000, ISO Guide 25, and Ministry of Health Accreditation.

Appendix C – Survey Plan



Surveyed & Drawn by:

 **BOUNDARY HUNTER LTD**

Ph. 09 435 5387

Email. bruce@boundaryhunter.co.nz

Northport Stormwater Pond
Existing Contour Survey

Prepared for:

Northport

Surveyed: BM

Checked: B Smith

Sheet:

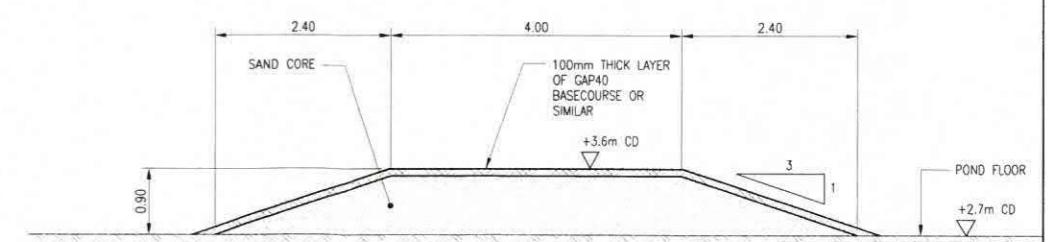
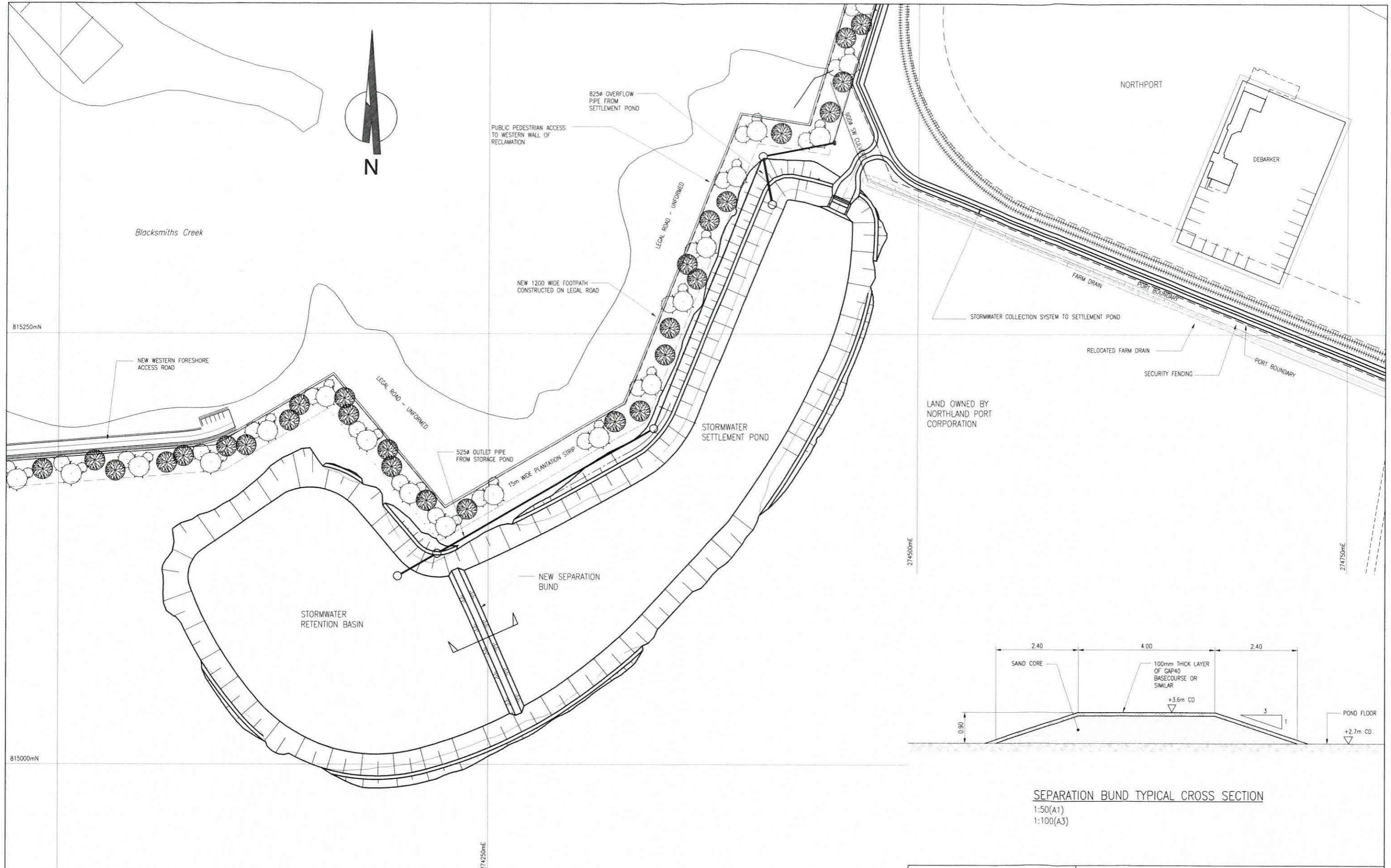
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Appendix D – Northport Stormwater Pond Plan and Detail

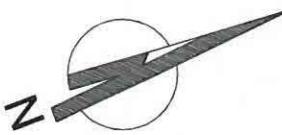


SEPARATION BUND TYPICAL CROSS SECTION

1:50(A1)
1:100(A3)

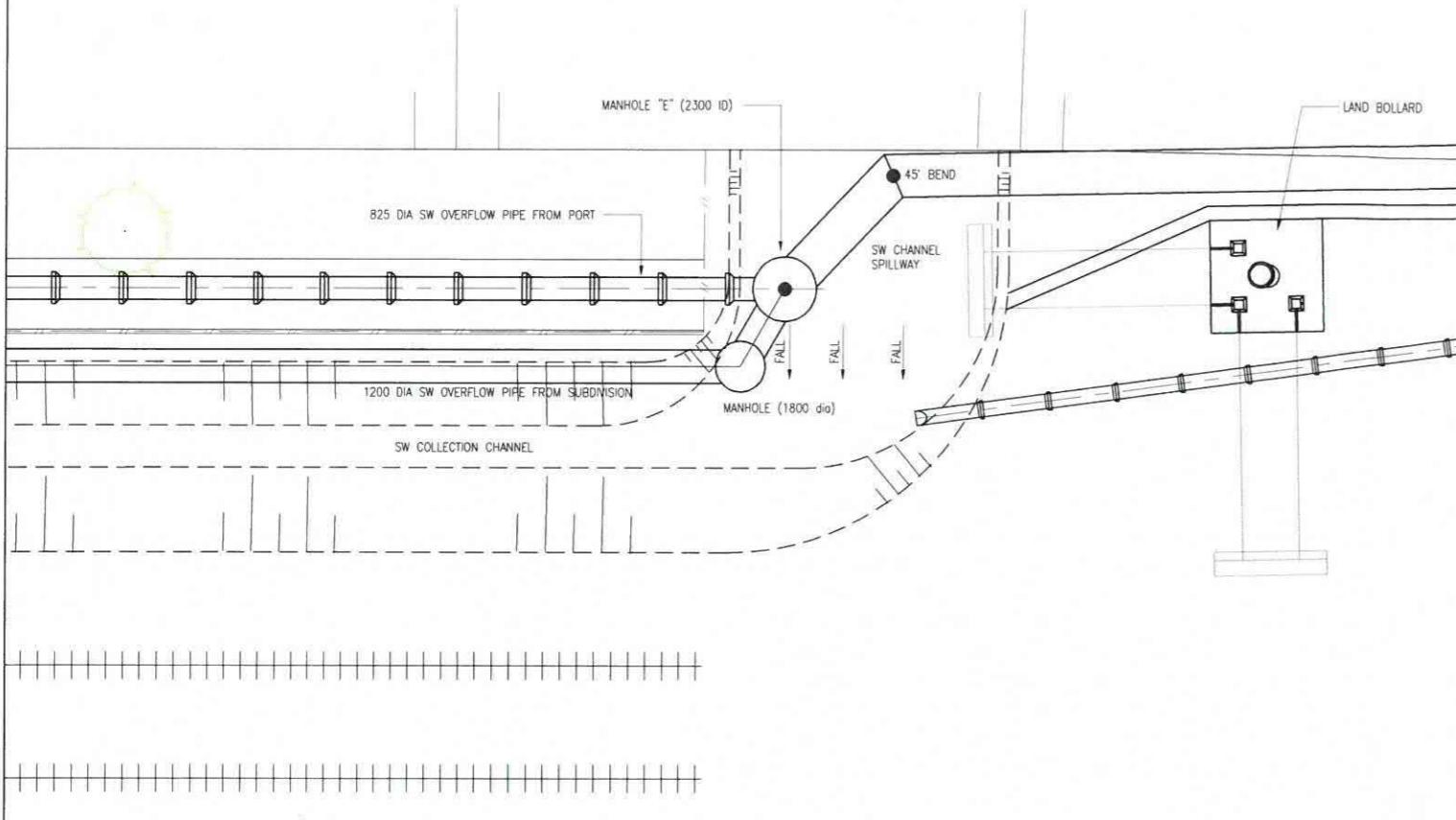
ISSUE	DATE	DETAILS	APPRVD BY
0	5.3.02	ISSUED	BWP

NOTE:
-COORDINATES ARE IN TERMS OF
GEOGRAPHIC DATUM 1949 MT EDEN CIRCUIT.
-LEVELS ARE IN TERMS OF CHART DATUM



SCHEDULE OF COORDINATES

45' BEND 816018.57mN 274579.81mE
CENTRE OF MANHOLE E 816013.51mN 274582.18mE



LOCALITY PLAN

1:125(A1)
1:250(A3)

0	20.8.02	ISSUED FOR PRICING	BWP
ISSUE	DATE	DETAILS	APPRVD

NOTE:
-COORDINATES ARE IN TERMS OF GEODETIC DATUM
1949, MT EDEN CIRCUIT.
-LEVELS ARE IN TERMS OF CHART DATUM.



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NORTHPORT DEVELOPMENT
STORMWATER CONTRACT
MANHOLE "E" DETAIL

Design By:	Date:	ITEM
	D60-30-06-01-012	0
Drawn By:	Check	Next
B PRESTON	AS SHOWN	AUGUST 2002

ORIGINAL SIZE A1

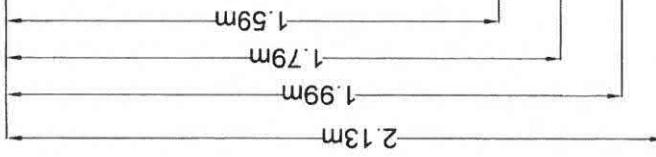
Appendix E – Pond Levels Schematic

NORTHPORT STORMWATER PUMP LEVELS

TOP OF DECK = RL 5.50m ▾

BOTTOM OF BEARER = RL 5.23m ▾

TOP OF PUMP RAIL = RL 4.77m ▾



MINIMUM PORT DATUM = RL 5.00m

TOP OF INLET WEIR = RL 4.00m

HIGH LEVEL ALARM = RL 3.64m ▾

START STANDBY PUMP = RL 3.44m ▾

START DUTY PUMP = RL 3.24m ▾

STOP BOTH PUMPS = RL 3.10m ▾

BASE OF POND = RL 2.70m ▾

Appendix F – Figure 01 – Overall Site Plan



TO BE READ ONLY IN CONJUNCTION WITH
ASSOCIATED REPORT

USE WRITTEN DIMENSIONS. DO NOT SCALE FROM DRAWING.

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SITE PLAN
SCALE - 1:5000

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CLIENT

NORTHPORT LTD
FUTURE DEVELOPMENT
PORT MARDSEN HIGHWAY, MARDSEN POINT
OVERALL SITE PLAN

PROJECT

DRAWING

SCALE @ A3 1:5000
PROJECT No. **12377**
FIGURE No. **01** REV. **R1**